Welcome to My Presentation

শিক্ষা নিয়ে গড়ব দেশ শেখ হাসিনার বাংলাদেশ।





Design of structure -1 (66463)

Md. Rezaul Bahar
Workshop Super & HoD (Civil)
Feni Polytechnic Institute,
Feni.

Cell Phone – 01712-623049 email- kajal.bahar@gmail.com



Design of Structure -1 (66463)

Design of Structure-1

Six semester, Diploma-in-engineering facebook live class-1

Important Chapters to discuss in online

- Design of Rectangular Beam (WSD Method)
- Design of Rectangular Beam (USD Method)
- Design of Doubly reinforced Beam
- Design of T- Beam
- Design of Lintel
- Design of Cantilever Beam

Introduction

- In structural design there are two important aspects —
- 1. Concretes
- 2. Reinforcements (Steel)
- Structures are made by R.C.C. In deferent structural safety & needs those two materials used in deferent strengths!
- In designing R.C.C structures we have to know the properties of concretes and steel (as reinforcements).

Concrete & Reinforcements classifications

Different types of Concrete –

- Plain Concrete
- R.C.C
- Pre-Stressed Concrete

Different types of Reinforcements –

- M.S. Bars
- 1. Plain Bar
- 2. Deformed bar
- 3. Twisted bar
- Pre-Stressing Steel

Image of different types of concrete



Plain Concrete



R.C.C



Pre-Stressed Concrete

Why M.S Rods choice as Reinforcements

- 1. Strength
- 2. Elasticity
- 3. Co-efficient of temperature expansion
- 4. Bond with concrete
- 5. Availability
- 6. Costing
- 7. Making in any shapes
- 8. Easy cut, bend, hook etc.

M.S bars (Reinforcements/Rebars)



Deformed bars



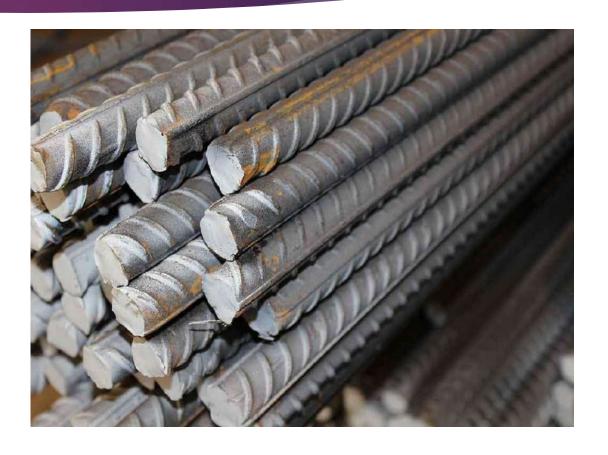
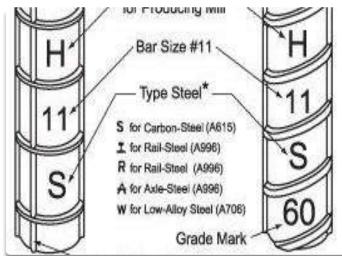


Image of different types Steel



Deformed bars



Types of steel



Tendon

Grades of Rebars

60 Grade bars



420 Metric Grade Bars

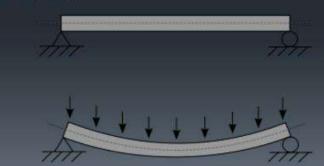


Design of Beam

Concept of Beam

What is a Beam?

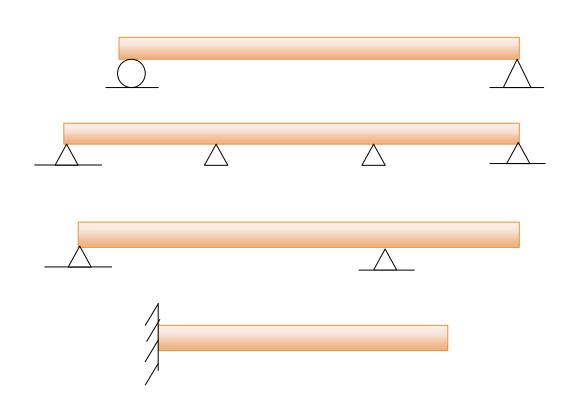
A Beam is any structural member which resists load mainly by bending. Therefore it is also called flexural member. Beam may be singly reinforced or doubly reinforced. When steel is provided only in tensile zone (i.e. below neutral axis) is called singly reinforced beam, but when steel is provided in tension zone as well as compression zone is called doubly reinforced beam.



Types of Beam

Types of Beam –

- 1. Simply Supported beam
- 2. Semi- Continuous beam
- 3. Fully Continuous beam
- 4. Over hanging beam
- 5. Cantilever beam



Flexural Assumptions

Flexural Assumptions-

- 1. Section remains plane
- 2. Stress proportional to Strain
- 3. Concrete not take tension
- 4. No concrete steel slip.

Working stress design method (WSD)

- This design concept is based on elastic theory, assuming a straight line stress distribution along the depth of the concrete. Concrete response elastically upto compressive strength not exceeding about ½ of its strength, while steel remains elastic practically upto yield strength. So, in practically, allowable stresses are set at about ½ the concrete compressive strength and ½ the yield stress of steel.
- The concrete remain elastic at ½×f'_c which range to strain of about 0.0005 and the steel is elastic near to it's yield point or strain of 0.002.
- According to ACI code the value is equal to 0.45× f'_c

Assumptions:

- 1) Section remains plane
- 2) Stress proportioned to Strain
- 3) Concrete not take tension
- 4) No concrete-steel slip

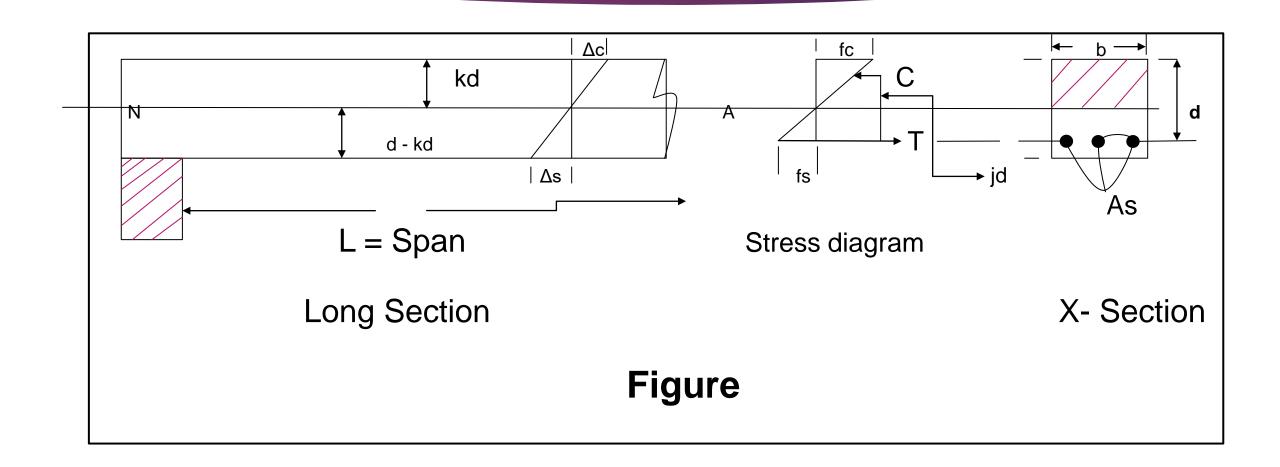
Notations

- 1. f'c = Ultimate strength of concrete
- 2. fc = Allowable stress of concrete
- 3. fy = Ultimate strength of steel
- 4. fs = Allowable stress of steel
- 5. $\Delta c = Strain of concrete$
- 6. $\Delta s = Strain of steel$
- 7. Es = Modulus of elasticity of steel
- 8. Ec = Modulus of elasticity of concrete
- 9. n= Modular ratio = $\frac{Es}{E_c}$

- 10. b = Width of beam
- 11. d = Depth of beam
- 12. L = Span of beam
- 13. $k = \frac{kd}{d}$
- $14. \quad j = \frac{jd}{d}$
- 15. jd = Lever arm
- 16. kd = Hight of compression zone
- 17. d-kd = Hight tension zone
- 18. $R = \frac{1}{2} f_c j k$

- 19. M= Bending moment
- 20. Mc = Resisting moment of concrete
- 21. Ms = Resisting moment of steel
- 22. C= Resultant of compressive forces
- 23. T= Resultant of tensile forces

Drive Flexural formulas: WSD Method



Flexural formulas

$$1. \quad k = \frac{n}{n + \frac{f_S}{f_C}}$$

2.
$$j = 1 - \frac{k}{3}$$

3.
$$d = \sqrt{\frac{M}{Rb}}$$

$$4. \quad A_S = \frac{M}{f_S j d}$$

$$5. \quad M_c = \frac{1}{2} f_c j k b d^{\nu}$$

6.
$$M_S = A_S f_S j d$$

7.
$$v = \frac{Vcr}{bd} \quad [V_{cr} = V - \frac{\omega d}{100}]$$

8.
$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v} \quad [v' = v - v_c]$$

8.
$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v} \quad [v' = v - v_c]$$
9. $S = \frac{A_v f_v}{v' b}$, $S = \frac{A_v}{0.0015b}$, $S = \frac{d}{2}$

10.
$$u = \frac{V}{\sum 0jd}$$
 $[u_{all} = \frac{3.23\sqrt{f_C'}}{D}]$

11.
$$n = \frac{Es}{Ec}$$

12.
$$f_c = 0.45 f'c$$

13.
$$f_S = 0 \cdot 4fy$$

14.
$$E_C = W^{1.5} 4270 \sqrt{f'} c$$

15.
$$E_s = 2039000 \, k \, g/c \, m^2$$

Home work: Derive these Flexural formulas and submit to your class teacher when your Institute will open.

If any question contact me!

As we discuss in this class, have any question?

কমেন্ট করে জানাও!

"এই ভিডিওটি পুণরায় দেখতে দক্ষতা বাতায়ন বা www.skills.gov.bd ভিজিট করুন!"



Practice in your house & welcome to next classes!

Thank You



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Design of Structure-1

6th semester, Chapter- 07

Steps of Rectangular Beam Design (WSD Method)

STRENGTH

doesn't come from what you

CAN do,

it comes from

OVERCOMING the things
you once thought you COULDN'T!.

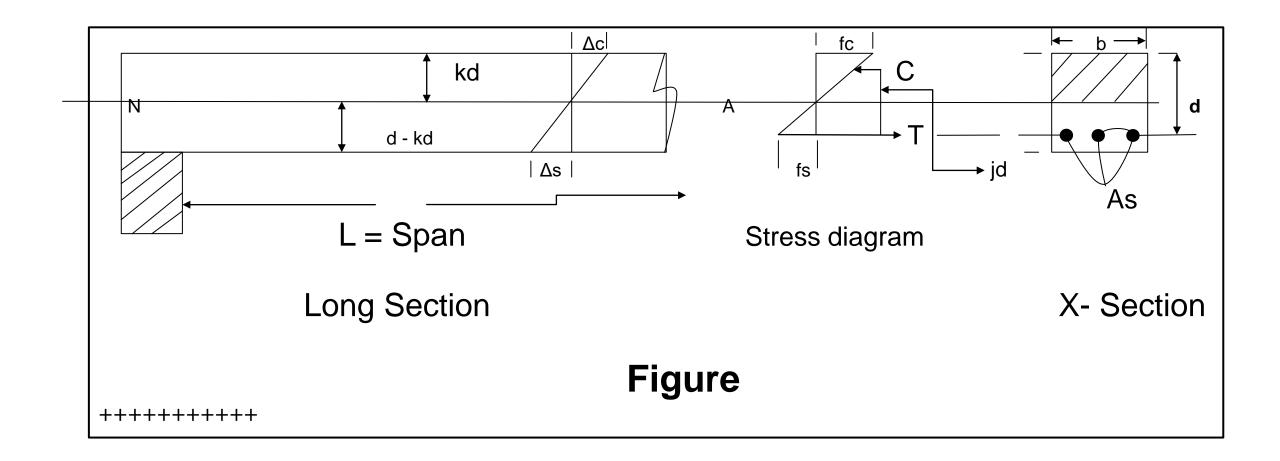
Recap

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- 2. f_c = Allowable stress of concrete
- 3. f_v = Ultimate strength of steel
- 4. f_s = Allowable stress of steel
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- 9. $n = \text{Modular ratio} = \frac{Es}{E_c}$

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Drive Flexural formulas: WSD Method



Flexural formulas

$$1. \quad k = \frac{n}{n + \frac{f_S}{f_C}}$$

2.
$$j = 1 - \frac{k}{3}$$

3.
$$d = \sqrt{\frac{M}{Rb}}$$

$$4. \quad A_S = \frac{M}{f_S j d}$$

$$5. \quad M_c = \frac{1}{2} f_c j k b d^{\nu}$$

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$$M_S = A_S f_S j d$$

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$$v = \frac{Vcr}{bd} \quad [V_{cr} = V - \frac{\omega d}{100}]$$

8.
$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v} \quad [v' = v - v_c]$$

9.
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, $S = \frac{A_v}{0.0015b}$, $S = \frac{d}{2}$

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$$E_C = W^{1.5} 4270 \sqrt{f'} c$$

15.
$$E_s = 2039000 \, k \, g/c \, m^2$$

Memorize these:

- \rightarrow 1 kg = 2.2046 Pounds
- ➤ I kip = 1000 Pounds
- $\rightarrow 1 \frac{kg}{cm^2} = 14.22 \text{ psi}$
- ► 1 MPa = 1 $\frac{N}{mm^2}$
- > 1 MPa = 145.0377 psi
- > 1 ksi = 6.8947 Mpa
- > 1 ksi = 1000 psi
- > 60 Grade = 420 Metric Grade

Step-1: Load Calculation

Given data -

 $f_C'/f_C =$

 $f_{S} =$

n =

L=

Super imposed load (L.L+ Others Loads except D.L) =

[Notations: L.L = Live Load, D.L= Dead Lod, Other Loads = Environmental load such as wind load, seismic load etc.]

Step-1: Load Calculation

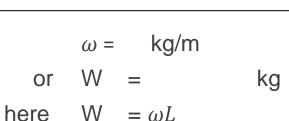
Let

d=10 % of L (or 4cm to 10cm per meter)

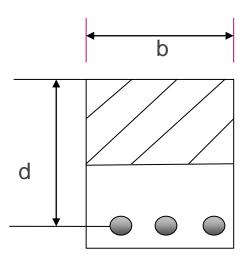
 $b = \frac{1}{2} to \frac{2}{3}$ of d $\left[\frac{1}{3} to \frac{3}{4} \text{ but not less tran 25 cm accordind to } BNBC\right]$

Super imposed load (L.L+ Others Loads) =

Self weight of Beam (D.L) = $L \times b \times d \times 2400$ =



[Note: if Super imposed load given in kg/m then L = 1 m]



Step-2: $V_{\text{max}} = Maximum Shear force$

$$V = \frac{W}{2} = \frac{\omega L}{2}$$
 for simply supported Beam

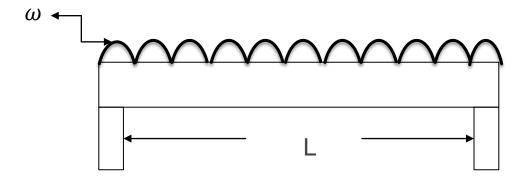
$$V = \frac{W}{2} = \frac{\omega L}{2}$$
 for fully Continuous Beam

$$V = 0.6 W = .6\omega L$$
 for semi- continuous Beam

(Continuous end)

$$V = 0.4 W = .4\omega L$$
 for semi- continuous Beam

(Dis-continuous end)



$$V = R = \frac{W}{2}$$

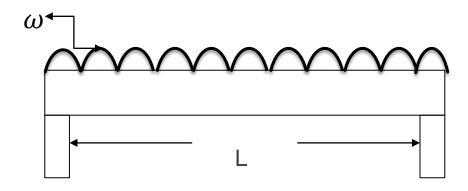
Step-3: $M_{\text{max}} = Maximum Bending Moment$

$$M = \frac{WL}{8} = \frac{\omega L^2}{8}$$
 kg.m for simply supported Beam

$$M = \frac{WL}{12} = \frac{\omega L^2}{12}$$
 kg.m for fully Continuous Beam

$$M = \frac{WL}{10} = \frac{\omega L^2}{10}$$
 kg.m for semi- continuous Beam

$$M = \frac{WL}{2} = \frac{\omega L^2}{2}$$
 kg.m for cantilever Beam



To covert those moments in kg.cm we have to multiply by 100

i.e. the formulas will like this $\mathbf{M} = \frac{\omega L^2}{8} \times 100 \text{ kg.cm}$. It will be apply for all beams.

Step-4: d = Effective depth

$$d = \sqrt{\frac{M}{Rb}}$$

$$R = \frac{1}{2}f_c jk \qquad [k = \frac{n}{n + \frac{f_s}{f_c}} \text{ and } j = 1 - \frac{k}{3}]$$

[Value ranges: k = 0.36 to 0.41, j = 0.86 to 0.89, R = 14 to 17]

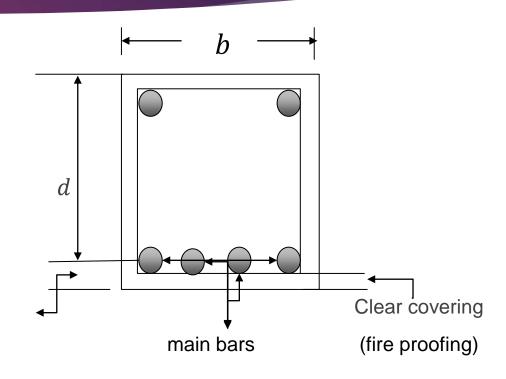
Total depth = d + total covering

Total covering

Total covering = $5 + \frac{\theta}{2}$ $\theta = Dia \ of \ main \ bar$

Size of Beam = $b \times Total depth$

$$\Rightarrow$$
 d = Total depth $-5 - \frac{\theta}{2}$ [here Clear covering = 5]



Step-5: A_s = Area of Steel

$$A_S = \frac{M}{f_S j d} \qquad cm^2$$

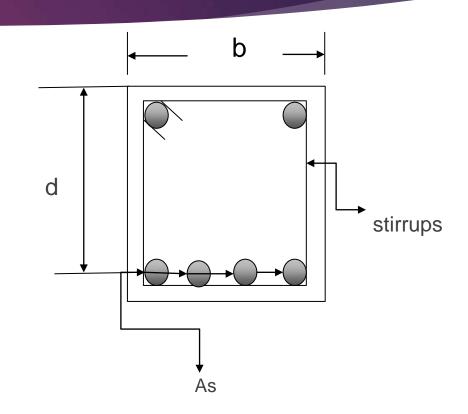
Find the total number of bars

By using area of bars.

Area =
$$\frac{\pi D^2}{4}$$

As a example area of 25 mm θ bar= 4.91 Cm^2

[Note d = Total depth
$$-5 - \frac{\theta}{2}$$
 [here Clear covering = 5]



- Home work:
- Memorize the area & perimeter of 10mm, 12mm, 16mm, 19 mm, 22mm, 25mm, 28mm, 32mm θ bar.

Dear students, have any question?

if any questions then comment here & contact with me!

আর -

এই ভিডিওটি পুণরায় দেখতে দক্ষতা বাতায়ন বা www.facebook.com/skills.gov.bd ভিজিট কর! LEARNING
is a TREASURE
that will follow its
OWNER everywhere!

Chinese Proverbs

Practice in your house & do the home works. welcome to next classes!

Thank You



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DESIGN OF STRUCTURE-1

6TH SEMESTER, CHAPTER- 07,

STEPS OF RECTANGULAR BEAM DESIGN (WSD METHOD) শিক্ষা নিয়ে গড়ব দেশ শেখ হাসিনার বাংলাদেশ।



Tell me and I will FORGET.
Show me and I may REMEMBER.
INVOLVE me and I will
UNDERSTAND!

Today's Lessons

6 Check for Shear Stress

7 Space required for Stirrups

8 Spacing of Stirrups

9 Check for Bond Stress

Detail Drawing

10

Recap

$$1. \quad k = \frac{n}{n + \frac{f_s}{f_C}}$$

2.
$$j = 1 - \frac{k}{3}$$

3.
$$d = \sqrt{\frac{M}{Rb}}$$

$$4. \quad A_S = \frac{M}{f_S j d}$$

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14.
$$E_C = W^{1.5}4270\sqrt{f'}c$$

15.
$$E_s = 2039000 \, k \, g/c \, m^2$$

Reminder:

Do you complete the home works?

If not, you are walking on the wrong track!

Change your attitude to be a successful one!

Step-1: Load Calculation

Given data -

 $f_C'/f_C =$

 $f_{S} =$

n =

L=

Super imposed load (L.L+ Others Loads except D.L) =

[Notations: L.L = Live Load, D.L= Dead Lod, Other Loads = Environmental load such as wind load, seismic load etc.]

Step-1: Load Calculation

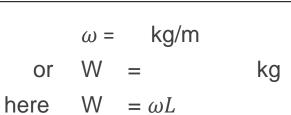
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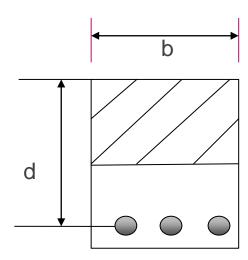
 $b = \frac{1}{2}to\frac{2}{3}$ of d $\left[\frac{1}{3}to\frac{3}{4} \text{ but not less tran 25 cm accordind to } BNBC\right]$

Super imposed load (L.L+ Others Loads) =

Self weight of Beam (D.L) = $L \times b \times d \times 2400$ =



[Note: if Super imposed load given in kg/m then L = 1 m]



Step-2: $V_{\text{max}} = Maximum Shear force$

$$V = \frac{W}{2} = \frac{\omega L}{2}$$
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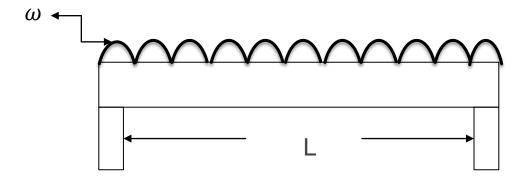
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(Dis-continuous end)



$$V = R = \frac{W}{2}$$

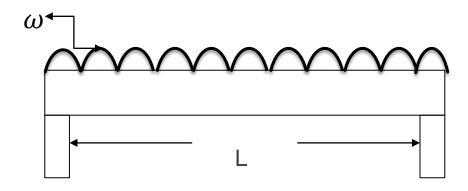
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 kg.m for simply supported Beam

$$M = \frac{WL}{12} = \frac{\omega L^2}{12}$$
 kg.m for fully Continuous Beam

$$M = \frac{WL}{10} = \frac{\omega L^2}{10}$$
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$$M = \frac{WL}{2} = \frac{\omega L^2}{2}$$
 kg.m for cantilever Beam



To covert those moments in kg.cm we have to multiply by 100

i.e. the formulas will like this $\mathbf{M} = \frac{\omega L^2}{8} \times 100 \text{ kg.cm}$. It will be apply for all beams.

Step-4: d = Effective depth

$$d = \sqrt{\frac{M}{Rb}}$$

$$R = \frac{1}{2}f_c jk \qquad [k = \frac{n}{n + \frac{f_s}{f_c}} \text{ and } j = 1 - \frac{k}{3}]$$

[Value ranges: k = 0.36 to 0.41, j = 0.86 to 0.89, R = 14 to 17]

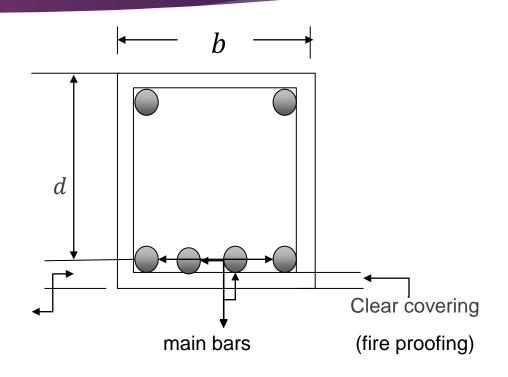
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Total covering

Total covering = $5 + \frac{\theta}{2}$ $\theta = Dia \ of \ main \ bar$

Size of Beam = $b \times Total depth$

$$\Rightarrow$$
 d = Total depth $-5 - \frac{\theta}{2}$ [here Clear covering = 5]



Step-5: A_s = Area of Steel

$$A_S = \frac{M}{f_S j d} \qquad cm^2$$

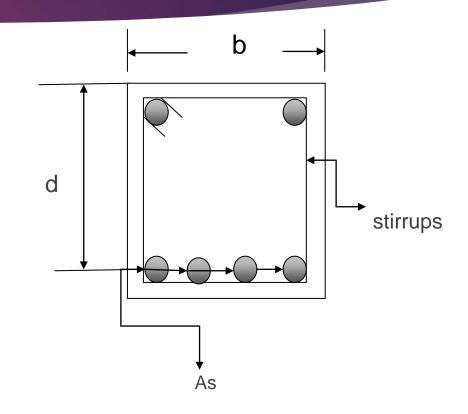
Find the total number of bars

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Area =
$$\frac{\pi D^2}{4}$$

As a example area of 25 mm θ bar= 4.91 Cm^2

[Note d = Total depth
$$-5 - \frac{\theta}{2}$$
 [here Clear covering = 5]



- Home work:
- Memorize the area & perimeter of 10mm, 12mm, 16mm, 19 mm, 22mm, 25mm, 28mm, 32mm θ bar.

Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{bd} \qquad [V_{cr} = V - \frac{\omega d}{100}]$$

here, $V_{cr} = V_d = V$ at d distance from support.

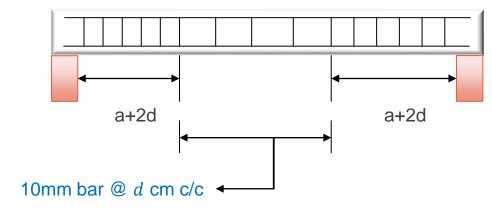
[100 is to convert d in meter]

[This is actual Shear stress ($v = v_{ac}$)]

But allowable stress of concrete is, $v_C = 0 \cdot 292\sqrt{f_C'}$

If $v > v_c$ stirrups required.

and access shear stress, $v' = v - v_c$



[Maximum shear stress of Beam = $1 \cdot 33\sqrt{f_C'}$, if $v < 1 \cdot 33\sqrt{f_C'}$ it will be safe in shear]

[Note: if $v < v_c$ then stirrups should be given at the spacing of d according to ACI code]

Step -7: Space required for stirrups, a

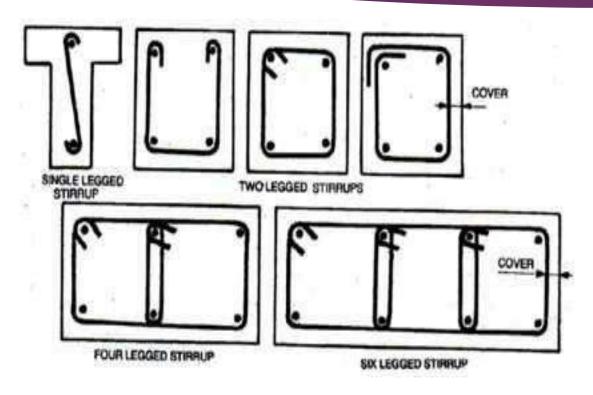
$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v}$$
 for simply supported Beam $a = \left(\frac{L}{2} - d\right) \frac{v'}{v}$ for fully continuous Beam $a = (0.6L - d) \frac{v'}{v}$ for semi - continuous Beam (at continuous end) $a = (0.4L - d) \frac{v'}{v}$ for semi - continuous Beam (at discontinuous end) $a = (L - d) \frac{v'}{v}$ for cantilever Beam

[Here access shear stress $v' = v - v_c$]

Total space required for stirrups = a + 2d [according to ACI]

[Note: Value of "L" should be in cm. because d is in cm.]

Different types of Stirrups







Stirrups details

Step-8: Spacing of Stirrups, S

1)
$$S = \frac{A_v f_v}{v'b} ,$$

2)
$$S = \frac{A_v}{0.0015b}$$
,

3)
$$s = \frac{d}{2}$$

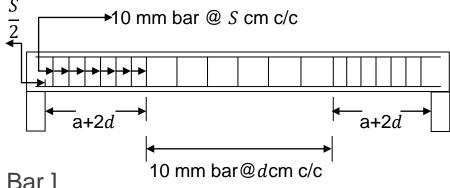
Here, $A_v = \text{Area of stirrups} = \text{Area of one stirrup x leg(s)}$

[normally stirrups are 10 mm dia. Bar]

 f_v = Allowable stress of stirrups

The minimum will be the Spacing of Stirrups.

[Note: Usually $f_v = f_s$]



Step-9: Check for Bond stress, u

$$u = \frac{V}{\sum 0 jd}$$
 Actual bond stress

Here, $\sum 0 = N\pi D$, D = Dia. Of main bars, N = Number of main bar

$$u_{all} = \frac{3 \cdot 23 \sqrt{f_C'}}{D}$$
 Allowable bond stress

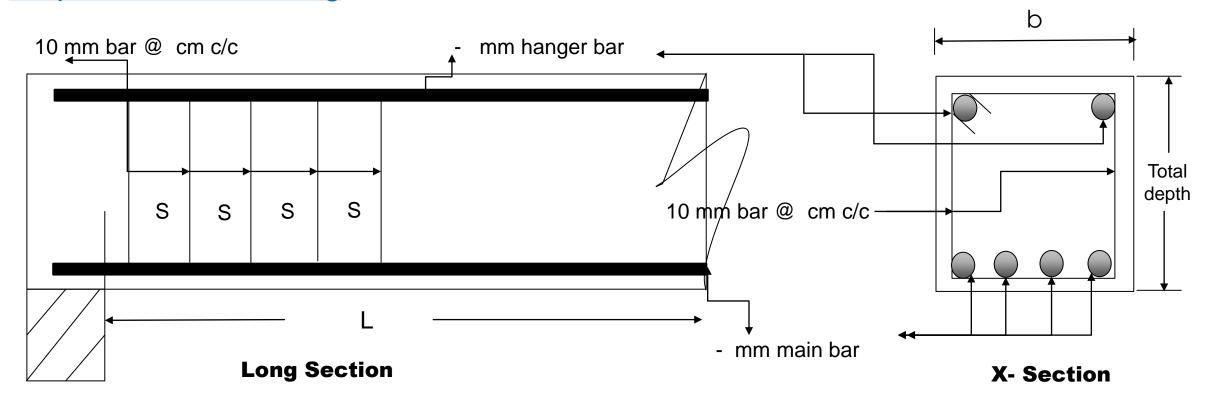
If $u < u_{all}$

safe in bond.

Home work: 1) Memorize all formulas from step-1 to step-9, 2) understand all steps of rectangular beam design (WSD Method) and try to design a beam from a example in your guide book! Submit to you Teacher after opening the institutions!

[N.B: One of the text book of this subject is Design of Concrete Structures by Arthur H. Nilson]

Step-10: Detail drawing



Practice the steps of beam design, if any question then comment & contact me!

আর -

এ**ই** ভিডিওটি পুণরায় দেখতে দক্ষতা বাতায়ন বা

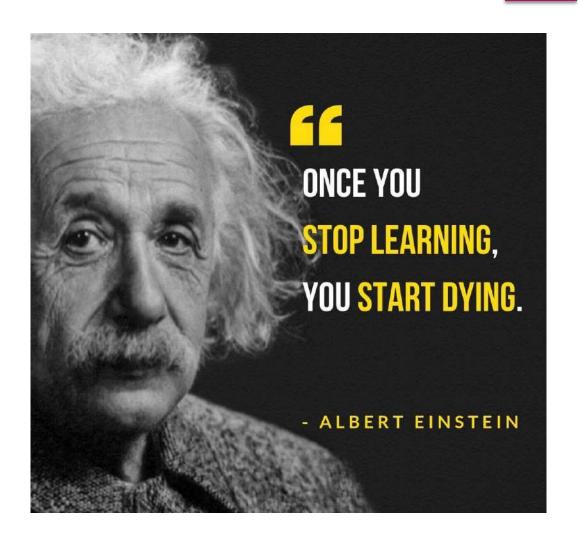
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ভিজিট কর !



Practice in your house & do the home works. welcome to next classes!

Thank You



Welcome to My Presentation

শিক্ষা নিয়ে গড়ব দেশ শেখ হাসিনার বাংলাদেশ।



Md. Rezaul Bahar

Workshop Super & HoD (Civil)

Feni Polytechnic Institute, Feni. email- kajal.bahar@gmail.com

DESIGN OF STRUCTURE-1

6TH SEMESTER (CIVIL), CHAPTER- 07, RECTANGULAR BEAM DESIGN (WSD METHOD)

TEACHRS open the door. You enter by yourself!



Remember

PROBLEM

When you trying to solve something, you will face some **PROBLEMS**!



SOLVE

By solving problems you will able to solve various kinds of PROBLEMS!



TRY YOUR BEST

Once you will solve the problem and you LEARN a lot, and also your will CONFIDENT!

IF YOU DON'T TRY

You can't **LEARN** and the way to **SUCCESS** will be blocked!



একটি সম্পূর্ণ অবিছিন্ন আয়তাকার বিমের ন্প্যান 5 মিটার l উহার উপর নিজস্ব ওজন ছাড়া প্রতি মিটারে 2500 কেজি লোড আরোপিত আছে l নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর l

```
f_c' = 210 \; k \; g/c \; m^2 , f_s = 1400 \; k \; g/c \; m^2 , n=10
```

Step-1: Load Calculation

Let

d=10 % of L = 50 cm

b = 25 cm

Super imposed load = 2500 kg/m

Self weight of Beam = $1 \times 0.25 \times 0.5 \times 2400 = 300 \text{ kg/m}$





$$V = \frac{\omega L}{2} = \frac{2800 \times 5}{2} = 7000 \text{ kg}$$

Step-3: $M_{\text{max}} = Maximum Bending Moment$

$$M = \frac{\omega L^2}{12} \times 100 = \frac{2800 \times 5^2}{12} \times 100 = 583334 \text{ kg.cm}$$



Step-4: *Effective depth, d*

$$d = \sqrt{\frac{M}{Rb}}$$

Here,

$$f_c = 0.45 f_c' = 0.45 \times 210 = 94.5 k g/c m^2$$

$$k = \frac{n}{n + \frac{f_s}{f_c}} = \frac{10}{10 + \frac{1400}{94 \cdot 5}} = 0.41, j = 1 - \frac{k}{3} = 1 - \frac{0.41}{3} = 0.86, R = \frac{1}{2}f_cjk = 16.66$$

$$d = \sqrt{\frac{M}{Rb}} = \sqrt{\frac{583334}{16.66 \times 25}} = 37.42 \text{ cm}$$

Total depth =
$$37.42 + 5 + \frac{2.5}{2} = 43.67 => 44 \text{ cm}$$
, [Total covering = $5 + \frac{2.5}{2}$]

Size of Beam = 25 cm x 44 cm.

hence
$$d = 44 - 5 - \frac{2.5}{2} = 37.75$$
 cm



Rebars area & perimeter

Number of Bar	In mm	Area in sq. cm	Perimeter in cm
#2	6	0.28	1.88
# 3	10	0.79	3.14
# 4	12	1.13	3.77
# 5	16	2.01	5.03
# 6	19	2.84	5.97
# 7	22	3.81	6.91
# 8	25	4.91	7.85
# 9	28	6.16	8.80
# 10	32	8.04	10.05
# 11	35	9.62	11.00

Step-5: A_S = Area of Steel

$$A_S = \frac{M}{f_S j d} = \frac{583334}{1400 \times .86 \times 37.75} = 12.84 \ cm^2$$

By using 2 - 22 mm dia bar + 2 - 19 mm dia bar the area is = 13.3 cm^2 > 12.84 cm^2 Use 2 - 22 mm Ø and 2 - 19 mm Ø bar as main bar.

Step - 6: Check for Shear stress, v

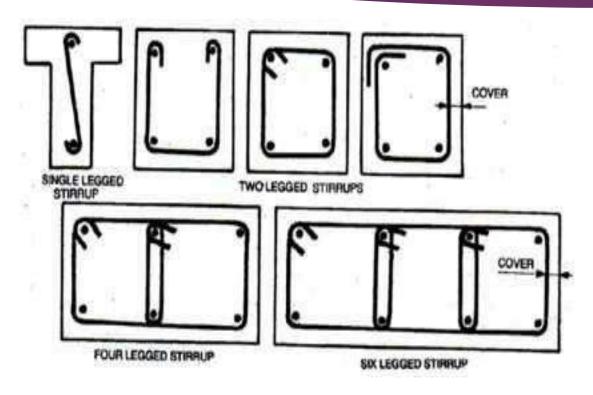
$$v = \frac{V_{cr}}{bd} = \frac{5943}{25 \times 37.75} = 6.30 \ kg/cm^2$$

Here, $V_{cr} = V - \frac{\omega d}{100} = 7000 - \frac{2800 \times 37.75}{100} = 5943 \text{ kg}$

But allowable stress of concrete is, $v_C = 0 \cdot 292 \sqrt{f_C'} = 4.23 \ k \ g/c \ m^2$ $v > v_C$ stirrups required.

and access shear stress, $v' = v - v_c$ = 6.30 - 4.23 = 2.07 $k g/c m^2$

Different types of Stirrups







Stirrups details

Step -7: Space required for stirrups, a

$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v} = \left(\frac{500}{2} - 37 \cdot 75\right) \frac{2.07}{6.30} = 62 \text{ cm}$$

Total space required for stirrups = $a + 2d = 62 + 2 \times 37.75 = 138$ cm

Step-8: Spacing of Stirrups, S

1)
$$S = \frac{A_v f_v}{v'b} = \frac{1.58 \times 1400}{2.07 \times 25} = 42.74$$
 cm Here, $A_v = \frac{\pi \times 1.0^2}{4} \times 2 = 1.58$ [Ring stirrups has 2 leg], $f_v = f_s$

2)
$$S = \frac{A_v}{0.0015b} = \frac{1.58}{0.0015 \times 25} = 42.13 \text{ cm}$$

3)
$$s = \frac{d}{2} = \frac{37.75}{2} = 18.87$$
 cm

The minimum spacing is $18.87 \text{ cm} \Rightarrow 18.5 \text{ cm}$

Use 10 mm Ø bar @ 18.5 cm c/c,

Step-9: Check for Bond stress, u

$$u = \frac{V}{\sum 0jd} = \frac{7000}{27.63 \times .86 \times 37.75} = 7.80 \ k \ g/c \ m^2 \qquad \sum 0 = N\pi D = 4 \times 3 \cdot 14 \times 2 \cdot 2 = 27.63$$

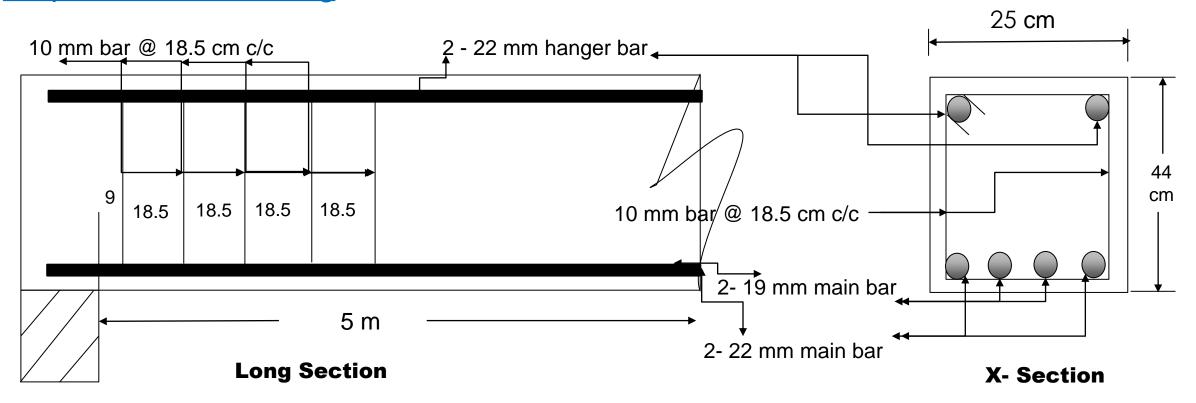
$$\sum 0 = N\pi D = 4 \times 3 \cdot 14 \times 2 \cdot 2 = 27.63$$

$$u_{all} = \frac{3.23\sqrt{f_c'}}{D} = \frac{3.23\times14.49}{2.2} = 21.27 \ k \ g/c \ m^2$$

$$u < u_{all}$$

safe in bond.

Step-10: Detail drawing



Home Work

1) একটি সাধারণভাবে স্থাপিত আয়তাকার বিমের ন্প্যান 5 মিটার l উহার উপর নিজস্ব ওজন সহ প্রতি মিটারে 3000 কেজি লোড আরোপিত আছে l নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর l

$$f_c' = 210 \; k \; g/c \; m^2$$
 , $f_s = 1400 \; k \; g/c \; m^2$, $n = 9$

2) একটি সাধারণভাবে স্থাপিত আয়তাকার বিমের ন্প্যান 6 মিটার l উহার উপর নিজস্ব ওজন ছাড়া প্রতি মিটারে 2500 কেজি লোড আরোপিত আছে l নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর l

$$f_c' = 210 \; k \; g/c \; m^2$$
 , $f_s = 1400 \; k \; g/c \; m^2$, $n = 9$

3) একটি আংশিক অবিছিন্ন আয়তাকার বিমের ন্প্যান 5.5 মিটার l উহার উপর নিজস্ব ওজন ছাড়া মোট 15400 কেজি লোড আরোপিত আছে l নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর l

$$f_c = 90 \ k \ g/c \ m^2$$
, $f_s = 1450 \ k \ g/c \ m^2$, $n=11$

Learning Progress

After design Lintel (WSD) Method

After design Doubly Beam (WSD) Method

After design T- Beam (WSD) Method

After design Rectangular Beam (USD) Method

After design Rectangular Beam (WSD) Method



You will learn 90% of Beam Design
You will learn 80% of Beam Design
You will learn 70% of Beam Design
You will learn 55% of Beam Design
You will learn 50% of Beam Design

Dear students, have any question? If any question comment here & contact with me!

আর -

এই ভিডিওটি পুণরায় দেখতে দক্ষতা বাতায়ন বা

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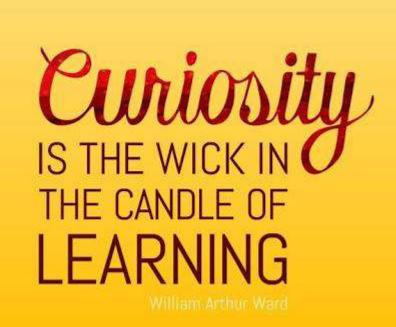
ভিজিট কর !

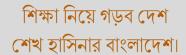
Practice at your home a lot!



Do the home works and practice enough. Welcome to next classes!

Thank You







Design of Structure-1

6th semester (Civil), Chapter- 08, Rectangular Beam Design (USD Method)

Presented by

Md. Rezaul Bahar

Workshop Super & HoD (Civil)

Feni Polytechnic Institute, Feni. email- kajal.bahar@gmail.com

USD METHOD

Today's Discussion

Difference of USD & WSD Method

Step-1
Load calculation

Notations used in USD method

Step-2

Maximum Shear force

Stress diagram of USD method

Step-3
Maximum Bending
Moment

Formulas used in of USD Method

Step-4
Effective depth

Difference between WSD and USD method

WSD (Working Stress Design) Method

It's based on the linear theory or elastic theory.

Its Consider to Design carrying load

Its Designing to plastic behavior of materials

Modular Ratio used for member Design

Stability of Structure is less then USD

High Cost Design Method

USD (Ultimate strength Design) Method

Its primarily based on strength concept of Concrete

Its Consider to Design Critical Combination of load

Its Designing to elastic behavior of materials.

Materials strength to be used for member Design.

Stability of Structure is more then WSD

Low Cost Design Method

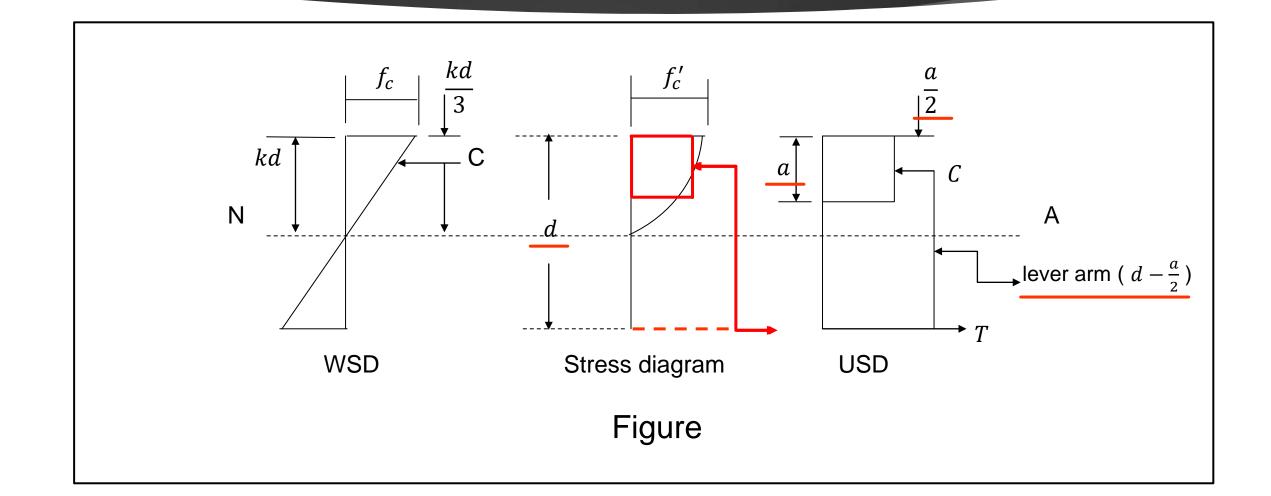
Notations

- 1. f_c' = Ultimate strength of concrete
- 2. f_y = Ultimate strength of steel
- 3. ε_c = Strain of concrete
- 4. $\varepsilon_{\rm S} = {\rm Strain} \ {\rm of} \ {\rm steel}$
- 5. Es = Modulus of elasticity of steel
- 6. Ec = Modulus of elasticity of concrete
- 7. b = Width of beam

- 8. d = Depth of beam
- 9. L = Span of beam
- 10. $\rho = \frac{A_s}{hd}$ = Steel ratio
- 11. ρ_b = Balanced steel ratio
- 12. $d \frac{a}{2} = \text{Lever arm}$
- 13. $M=M_u$ = Bending moment
- 14. M_n = Nominal resisting moment
- 15. $a = \frac{A_s f_y}{0.85 f_C' b}$
- *16.* φ = Reduction factor

- *17. L.L*= Live Load
- 18. D.L = Dead Load
- 19. W.L = Wind Load
- 20. E = Earthquake
- 21. γ_L = Load factor for L.L
- 22. γ_D = Load factor for D.L
- 23. γ_w = Load factor for W.L
- 24. γ_E = Load factor for Seismic load

Drive Flexural formulas: USD Method



Flexural formulas

1.
$$M_n = \rho f_y b d^2 \left(1 - .59 \frac{\rho f_y}{f_i} \right)$$

2.
$$M_n = \emptyset \rho f_y b d^2 \left(1 - .59 \frac{\rho f_y}{f_i} \right)$$

$$3. \quad M_n = A_s f_{\gamma} \left(d - \frac{a}{2} \right)$$

$$4. \quad M_u = \emptyset A_s f_{\gamma} \left(d - \frac{a}{2} \right)$$

5.
$$C = .85 f_c' ab$$

6.
$$T = A_s f y$$

7.
$$z = Leverarm = d - \frac{a}{2}$$

8.
$$a = \frac{A_s f_y}{.85 f_C' b}$$

9.
$$\rho = \frac{A_S}{bd}$$

10.
$$\rho_b = .85\beta_1 \times \frac{f_c'}{fy} \times \frac{6117}{6117 + f_y}$$

11.
$$\rho_{\text{max}} = \rho = 0.75 \ \rho_b$$

12.
$$M_n = \rho f_y b d^2 \left(1 - .59 \frac{\rho f_y}{f'c} \right)$$

13.
$$M_u = \phi \rho f_y b d^2 \left(1 - .59 \frac{\rho f_y}{f'c} \right)$$

14.
$$C = .85 f_c' ab$$

15.
$$T = A_s f_y$$

16. Lever =
$$z = d - \frac{a}{2}$$

17.
$$A_s = \rho b d$$

18.
$$d = \sqrt{\frac{M}{\emptyset^{\rho} f_{y} b \left(1 - 0.59 \rho \frac{fy}{f'c}\right)}}$$

Home work:

Drive the formula number 8 & 18

Step-1: Load Calculation

Given data -

 $f_c' =$

 $f_y =$

L=

L.L=

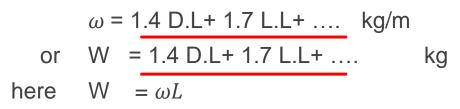
Other loads =

[Notations: L.L = Live Load, D.L= Dead Lod, Other Loads = Environmental load such as wind load, seismic load etc.]

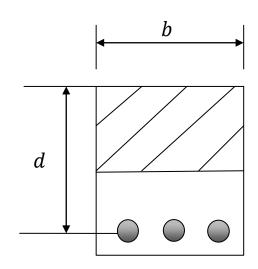
Step-1: Load Calculation

Let

d=10 % of L (or 4cm to 10cm per meter)
$$b = \frac{1}{2}to\frac{2}{3} \text{ of d } \left[\frac{1}{3}to\frac{3}{4} \text{ but not less tran 25 cm accordind to BNBC}\right]$$
L.L =
$$Self \text{ weight of Beam (D.L)} = L \times b \times d \times 2400 =$$



[Note: if Super imposed load given in kg/m then L = 1 m]



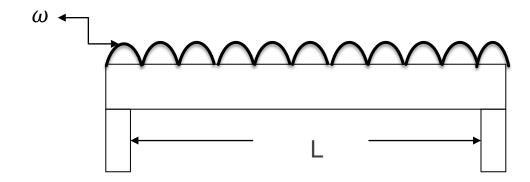
Step-2: $V_{\text{max}} = Maximum Shear force$



$$V = \frac{W}{2} = \frac{\omega L}{2}$$
 for fully Continuous Beam

(Continuous end)

(Dis-continuous end)



$$V = R = \frac{W}{2}$$

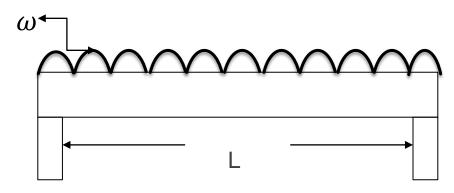


$$M = \frac{WL}{8} = \frac{\omega L^2}{8}$$
 kg.m for simply supported Beam

$$M = \frac{\omega L^2}{12}$$
 kg.m for fully Continuous Beam

$$M = \frac{\omega L^2}{10}$$
 kg.m for semi- continuous Beam

$$M = \frac{\omega L^2}{2}$$
 kg.m for cantilever Beam



To covert those moments in kg.cm we have to multiply by 100

i.e. the formulas will like this $\mathbf{M} = \frac{\omega L^2}{8} \times 100$ kg.cm. It will be apply for all beams.

Step-4: d = Effective depth

$$d = \sqrt{\frac{M}{\emptyset^{\rho} f_{y} b \left(1 - 0.59 \rho \frac{f y}{f' c}\right)}}$$

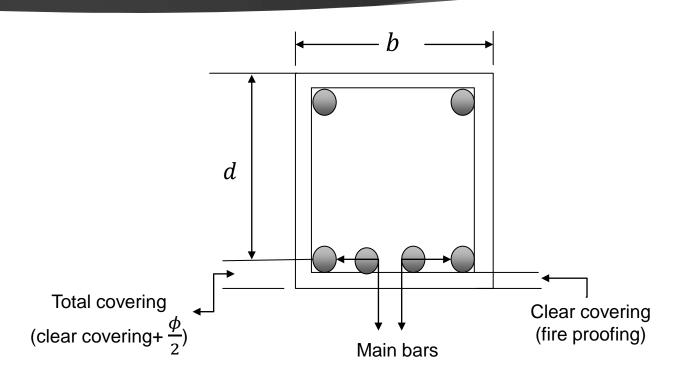
$$\emptyset = 0.9$$

Total depth = d + total covering

Total covering = $5 + \frac{\theta}{2}$ $\theta = Dia \ of \ main \ bar$

Size of Beam = $b \times Total depth$

=> d = Total depth – 5 - $\frac{\theta}{2}$ [here Clear covering = 5]



Dear students, do you understand USD method?

if any questions then comment here & contact with me!

আর -

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ভিজিট কর !

Practice in your house & do the home works. welcome to next classes!

Thank You

Stay Home, Stay Safe







শিক্ষা নিয়ে গড়ব দেশ শেখ হাসিনার বাংলাদেশ।



Design of Structure-1

6th semester (Civil), Chapter- 08, Rectangular Beam Design (USD Method)

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Today's Discussion

Area of Steel

Check for Shear stress

STEP O7 Space required for Stirrups

STEP Spacing of Stirrups

Check for Bond stress

Detail Drawing

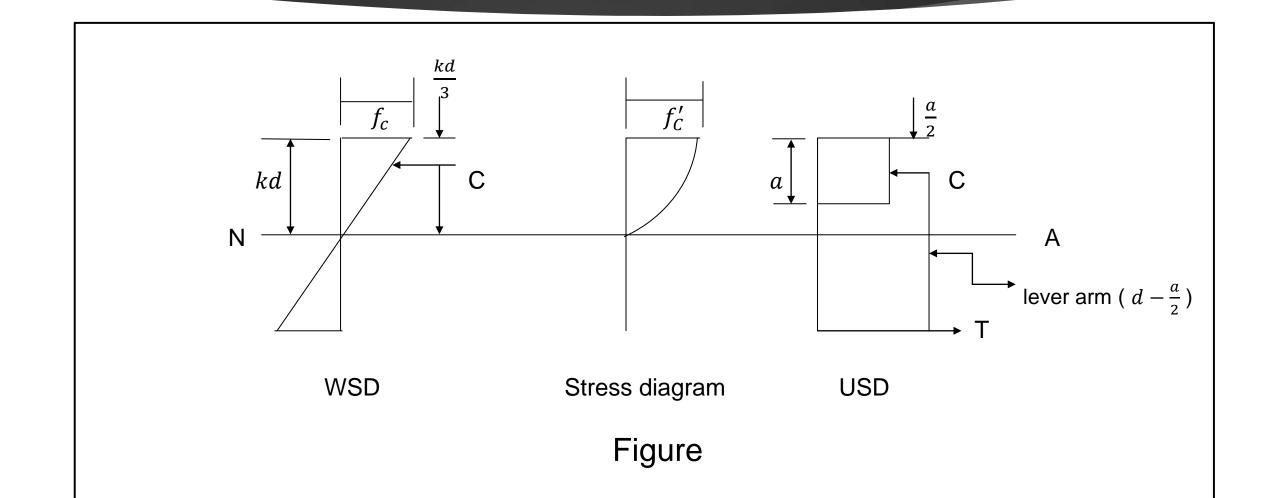
Notations

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- 4. $ε_5$ = Strain of steel
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- 14. M_n = Nominal resisting moment
- 15. $a = \frac{A_S f_y}{0.85 f_C' b}$
- 16. φ= Reduction factor

- 17. L.L= Live Load
- 18. D.L = Dead Load
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Drive Flexural formulas: USD Method



Flexural formulas

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$$M_n = \rho f_y b d^2 \left(1 - .59 \frac{\rho f_y}{f_i} \right)$$

2.
$$M_n = \emptyset \rho f_y b d^2 \left(1 - .59 \frac{\rho f_y}{f_i} \right)$$

$$3. \quad M_n = A_s f_{\gamma} \left(d - \frac{a}{2} \right)$$

$$4. \quad M_u = \emptyset A_s f_{\gamma} \left(d - \frac{a}{2} \right)$$

5.
$$C = .85 f_c' ab$$

6.
$$T = A_s f y$$

7.
$$z = Leverarm = d - \frac{a}{2}$$

8.
$$a = \frac{A_s f_y}{.85 f_C' b}$$

9.
$$\rho = \frac{A_S}{bd}$$

10.
$$\rho_b = .85\beta_1 \times \frac{f_c'}{f_y} \times \frac{6117}{6117 + f_y}$$

11.
$$\rho_{\text{max}} = \rho = 0.75 \ \rho_b$$

12.
$$M_n = \rho f_y b d^2 \left(1 - .59 \frac{\rho f_y}{f'c} \right)$$

13.
$$M_u = \phi \rho f_y b d^2 \left(1 - .59 \frac{\rho f_y}{f'c} \right)$$

14.
$$C = .85f_c'ab$$

15.
$$T = A_s f_y$$

16. Lever =
$$z = d - \frac{a}{2}$$

17.
$$A_s = \rho bd$$

18.
$$d = \sqrt{\frac{M}{\phi^{\rho} f_{y} b \left(1 - 0.59 \rho \frac{fy}{f'c}\right)}}$$

Home work:

Drive the formula number 8 & 18

Step-5: A_s = Area of Steel

$$A_{s} = \rho b d \qquad cm^{2}$$

$$\rho = \rho_{\text{max}} = 0.75 \ \rho_{b}$$

$$\rho_{b} = .85 \times \beta_{1} \times \frac{f'_{c}}{fy} \times \frac{6117}{6117 + fy}$$

 β_1 = 0.85 if $f_c' \le$ 281 $K_g/c m^2$ [It will 0.05 less for every

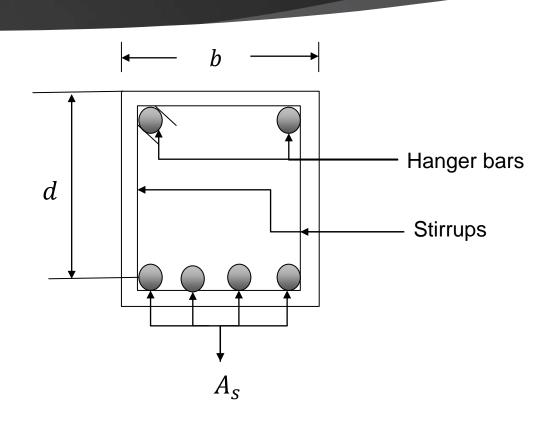
70 $K_g/c m^2$ but not less than 0.65]

Find the total number of bars

By using area of bars.

Area =
$$\frac{\pi D^2}{4}$$

[Note d = Total depth $-5 - \frac{\theta}{2}$ [here Clear covering = 5]



Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{bd} \qquad [V_{cr} = V - \frac{\omega d}{100}]$$

here, $V_{Cr} = V_d = V$ at d distance from support.

[100 is to convert d in meter]

This is actual Shear stress ($v = v_{ac}$)

But allowable stress of concrete is, $v_C = \phi \ 0.53 \sqrt{f_C'} \ [\phi = 0.85]$

If $v > v_c$ stirrups required.

and access shear stress, $v' = v - v_c$

[Note: Maximum shear stress of beam = $2 \cdot 65\sqrt{f_c'} \ cm^2$]

Step -7: Space required for stirrups, a

$$a=\left(\frac{L}{2}-d\right)\frac{v'}{v}$$
 for simply supported Beam $a=\left(\frac{L}{2}-d\right)\frac{v'}{v}$ for fully continuous Beam $a=(0.6L-d)\frac{v'}{v}$ for semi - continuous Beam (at continuous end) $a=(0.4L-d)\frac{v'}{v}$ for semi - continuous Beam (at discontinuous end) $a=(L-d)\frac{v'}{v}$ for cantilever Beam [Here access shear stress $v'=v-v_c$]

Total space required for stirrups = a + 2d [according to ACI]

[Note: Value of "L" should be in cm.]

Step-8: Spacing of Stirrups, S

$$S = \frac{\phi A_v f_v}{v'b}$$

2)
$$S = \frac{d}{2}$$

3)
$$S = 60 cm$$

Here, $A_v =$ Area of stirrups = Area of one stirrup x leg(s) [normally stirrups are 10 mm dia. Bar] $f_v =$ Allowable stress of stirrups $\phi = 0.85$

The minimum will be the Spacing of Stirrups.

Step-9: Check for Bond stress, u

$$u = \frac{V}{\Phi \sum 0(d - \frac{a}{2})}$$
 Actual bond stress

Here, $\sum 0 = N\pi D$, D = Dia. of main bars, N = Number of main bar, $Jd = d - \frac{a}{2}$, $\phi = 0.85$

$$u_{all} = \frac{6.39\sqrt{f_C'}}{D}$$
 Allowable bond stress

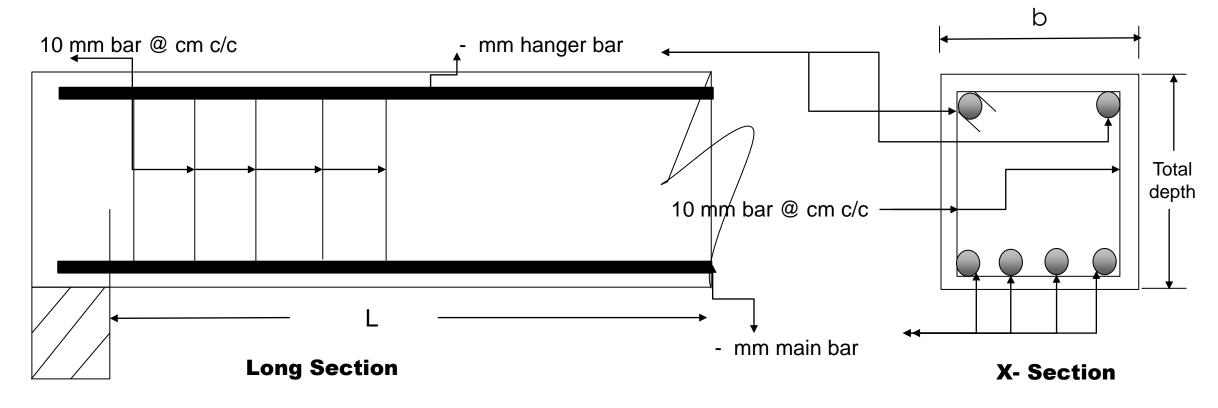
If $u < u_{all}$

safe in bond.

Home work: 1) Memorize all formulas from step-1 to step-9, 2) understand all steps of rectangular beam design (USD Method) and try to design a beam from a example in your guide book! Submit to you Teacher after opening the institutions!

[N.B: One of the text book of this subject is Design of Concrete Structures by Arthur H. Nilson]

Step-10: Detail drawing



Dear students, do you understand USD method?

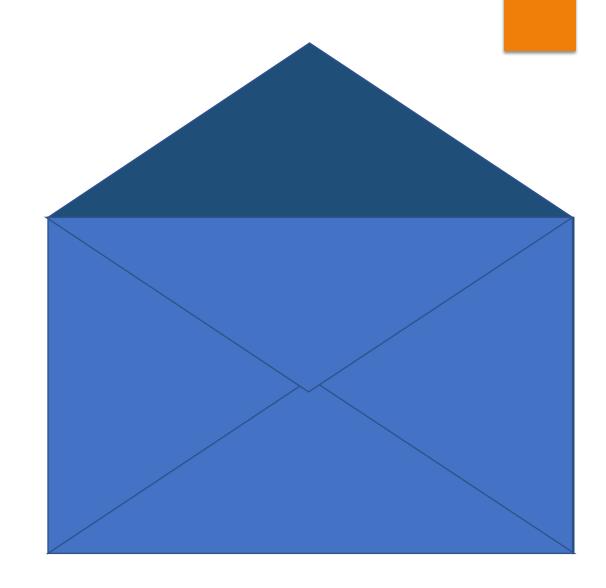
if any questions then comment here & contact with me!

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welcome to next classes!

Thank You

Practice in your house & do the home works.



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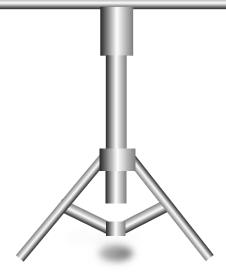
Design of Structure-1

6th semester (Civil), Chapter- 08, Rectangular Beam Design (USD Method)

Presented by: Md. Rezaul Bahar

Workshop Super & HoD (Civil)

Feni Polytechnic Institute, Feni. email- kajal.bahar@gmail.com





একটি সম্পূর্ণ অবিছিন্ন বিমের স্প্যান 6 মিটার | উহার উপর প্রতি মিটার দৈর্ঘ্যে নিজস্ব ওজন সহ 2000 কেজি ডেড লোড ও 2200 কেজি লাইভ লোড আরোপিত আছে | নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর | তথ্যাদি-

$$f_c' = 280 \ k \ g/c \ m^2$$
 , $f_y = 4000 \ k \ g/c \ m^2$

Step-1: Load Calculation

Let

$$d = 50 \text{ cm}$$

$$b = 25 \text{ cm}$$

 $\omega = 1.4 \text{ D.L+ } 1.7 \text{ L.L} = 1.4 \times 2000 + 1.7 \times 2200 = 6540 \text{ kg/m}$

Step-2:
$$V_{\text{max}} = Maximum Shear force$$

$$V = \frac{\omega L}{2} = \frac{6540 \times 6}{2} = 19620 \text{ kg}$$

Step-3: $M_{\text{max}} = Maximum \ Bending \ Moment$

$$M = \frac{\omega L^2}{12} \times 100 = \frac{6540 \times 6^2}{12} \times 100 = \frac{19620000}{12} \text{ kg.cm}$$

Step-4: d = Effective depth

$$d = \sqrt{\frac{M}{\phi^{\rho} f_{y} b \left(1 - 0.59 \rho \frac{fy}{f'c}\right)}} \qquad [\underline{\rho_{b}} = .85 \beta_{1} \times \frac{f'_{c}}{fy} \times \frac{6117}{6117 + f_{y}} = .85 \times .85 \times \frac{280}{4000} \times \frac{6117}{6117 + 4000} = \underline{0.03}, \ \underline{\rho} = 0.75 \times 0.03 = \underline{0.0225}]$$

$$= \sqrt{\frac{19620000}{.9 \times .0225 \times 4000 \times 25 \left(1 - .59 \times .0225 \times \frac{4000}{280}\right)}} = 36.01 \text{ cm}, \text{ Total depth} = 36.01 + 6.25 = 42.26 => 43 \text{ cm}.$$

The size of beam = 25 cm x 43 cm.

$$d = 43 - 6.25 = 36.75$$
 cm

Step-5: A_s = Area of Steel

 $A_s = \rho bd = 0.0225 \times 25 \times 36.75 = 20.67 cm^2$

By using 2 nos. 28mm + 2 nos. 25 mm bar the area = 22.14 cm^2 >20.67 cm^2

Use 2 - 28 mm + 2 - 25 mm ϕ bar as main bars.

Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{bd} = \frac{17216.55}{25 \times 36.75} = 18.74 \ cm^2 \qquad [V_{cr} = 19620 - \frac{6540 \times 36.75}{100} = 17216.55]$$

Allowable stress of concrete is, $v_C = \phi \ 0.53\sqrt{f_C'} = 0.85 \ \text{x} \ 0.53 \ \text{x} \ \sqrt{280} = \underline{7.54 \ cm^2}$

 $v > v_c$ stirrups required.

and access shear stress, $v' = v - v_c$ = 18.74 – 7.54 = 11 cm^2

Step -7: Space required for stirrups, a

$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v} = \left(\frac{600}{2} - 36 \cdot 75\right) \frac{11}{18 \cdot 74} = 155 \text{ cm}$$

Total space = $a + 2d = 155+2 \times 36.75 = 229 \text{ cm} = 2.28 \text{ m}$

Step-8: Spacing of Stirrups, S

1)
$$S = \frac{\phi A_v f_v}{v'b} = \frac{.85 \times 1.58 \times 4000}{11 \times 25} = 19.53 \text{ cm}$$
 [$A_v = 0.5$

[
$$A_v = 0.79 \times 2 = 1.58 \text{ } cm^2$$
, $f_v = 4000$, $v' = 11$, $\phi = 0.85$]

2)
$$S = \frac{d}{2} = \frac{36.75}{2} = 18.37 \text{ cm}$$

3) S = 60 cm

The minimum Spacing of Stirrups is 18.37 cm => 18 cm

Use 10 mm ϕ bar @ 18 cm c/c as stirrups.

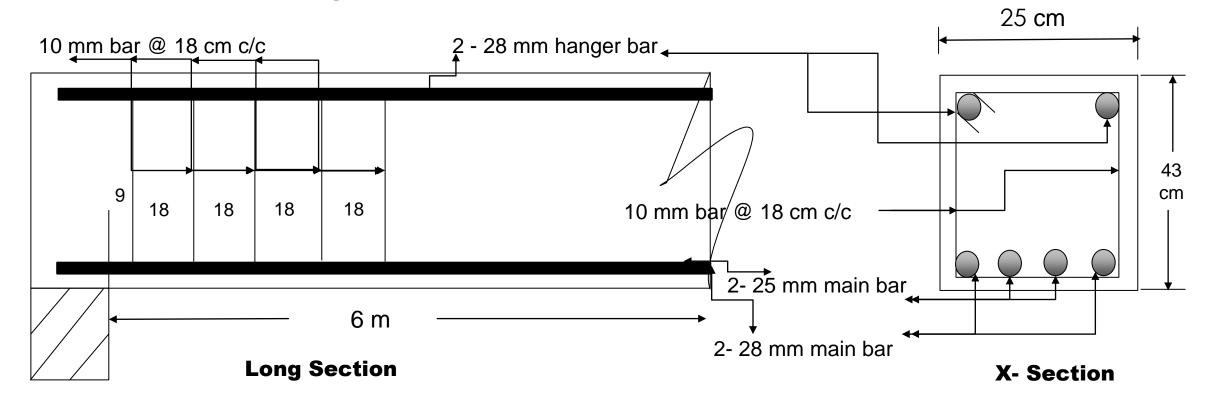
Step-9: Check for Bond stress, u

$$u = \frac{V}{\Phi \sum 0(d - \frac{a}{2})} = \frac{19620}{.85 \times 31 \cdot 4 \times 29 \cdot 31} = \frac{25.08 \text{ cm}^2}{.85 \times 31 \cdot 4 \times 29 \cdot 31} = \frac{25.08 \text{ cm}^2}{.85 \times 280 \times 25} = \frac{22 \cdot 14 \times 4000}{.85 \times 280 \times 25} = \frac{14.88}{.85 \times 280 \times 25} =$$

$$u_{all} = \frac{6.39\sqrt{f_C'}}{D} = \frac{6.39\times\sqrt{280}}{2.8} = \frac{38.18 \text{ cm}^2}{2.8}$$
 [$\sum 0 = N\pi D = 4\times3\cdot14\times2\cdot5 = 31.4 \text{ cm}$]

 $u < u_{all}$, hence safe in bond stress.

Step-10: Detail drawing



Home Work

- 1) একটি সাধারণ ভাবে স্থাপিত বিমের স্প্যান 5 মিটার | উহার উপর প্রতি মিটার দৈর্ঘ্যে নিজস্ব ওজন ছাড়া 2000 কেজি ডেড লোড ও 2500 কেজি লাইভ লোড আরোপিত আছে | নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর | তথ্যাদি- $f_c' = 300 \ k \ g/c \ m^2$, $f_v = 4200 \ k \ g/c \ m^2$
- 2) একটি আংশিকঅবিছিন্ন বিমের স্প্যান 6 মিটার | উহার উপর প্রতি মিটার দৈর্ঘ্যে নিজস্ব ওজন সহ 2200 কেজি ডেড লোড ও 2500 কেজি লাইভ লোড আরোপিত আছে | নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর | তথ্যাদি- $f_c' = 280 \ k \ g/c \ m^2$, $f_v = 4200 \ k \ g/c \ m^2$
- 3) একটি সম্পূর্ণ অবিছিন্ন বিমের স্প্যান 5 মিটার l উহার উপর প্রতি মিটার দৈর্ঘ্যে নিজস্ব ওজন সহ 2250 কেজি ডেড লোড ও 2500 কেজি লাইভ লোড আরোপিত আছে l নিম্নের তথ্যাদির সাহায্যে বিমটির আকার, স্টিলের পরিমান ও ডিজাইন কর l তথ্যাদি-

 $f_c' = 280 \ k \ g/c \ m^2$, $f_v = 4200 \ k \ g/c \ m^2$

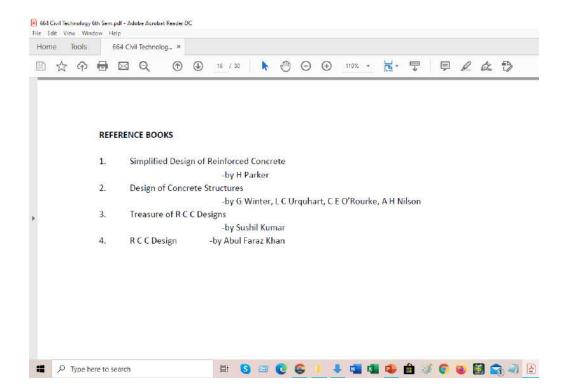
Reference Books of Design of Structure-1

Link for download syllabus from BTEB (<u>www.bteb.gov.bd</u>) website (সিলেবাস ডাউনলোড লিংক)

https://drive.google.com/drive/folders/1tDUHSkToHSyVj2OoWTI9He1P-pAe56BQ

Reference Books of Design of Structure-1

- 1. Simplified Design of Reinforced Concrete -by H Parker
- Design of Concrete Structures -by G Winter, L C Urquhart, C E O'Rourke, A H Nilson
- 3. Treasure of R C C Designs -by Sushil Kumar
- 4. R C C Design -by Abul Faraz Khan



Dear students, do you understand USD method?

if any questions then comment here & contact with me!

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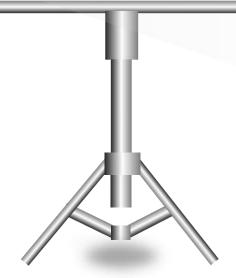
Design of Structure-1

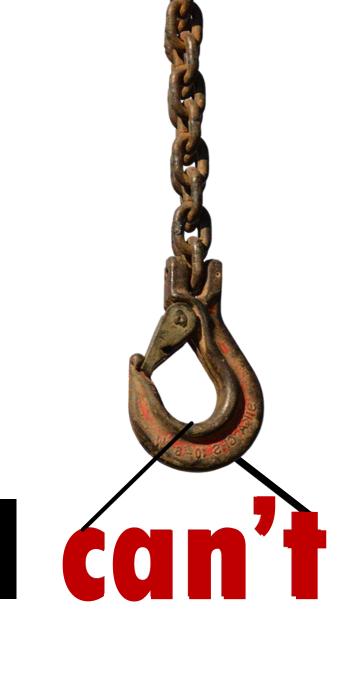
6th semester (Civil), Chapter- 10, T- Beam Design (WSD Method)

Presented by: Md. Rezaul Bahar

Workshop Super & HoD (Civil)

Feni Polytechnic Institute, Feni. email- kajal.bahar@gmail.com

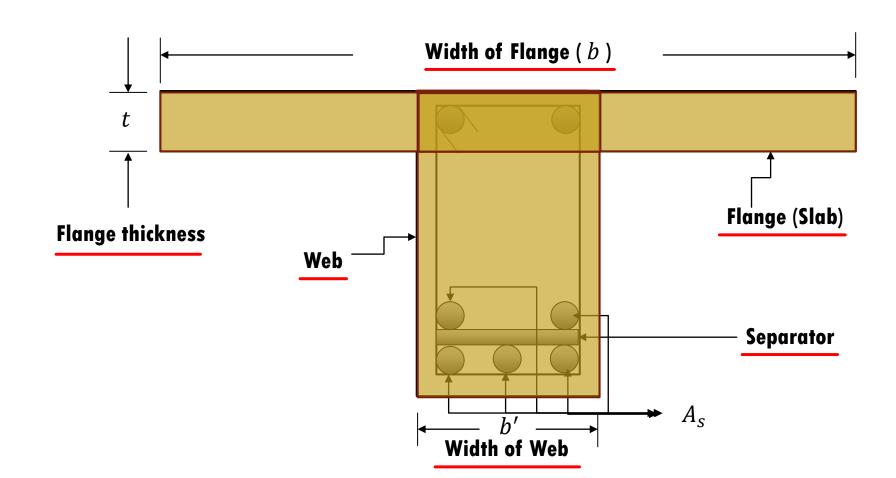




Be Positive to be a Successful!

I can't Understand!

T - BEAM



Concept of T- Beam

- T- Beam gives more room height
- The Web provide greater separation for bending
- The Web resist shear stress
- The Flange resist compressive stresses.
 - T- Beam design concept

T- Beam

Selection of b_E

Interior T-section:

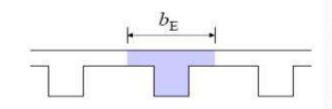
- 1) $b_{\rm E} \leq L/4$
- 2) $b_{\rm E} \le b_{\rm w} + 16t$
- 3) $b_{\rm E} \le \text{c-c}$ of beams

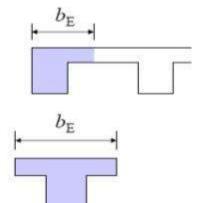
Exterior T-section:

- 1) $b_{\rm E} \leq b_{\rm w} + L / 12$
- $2)~b_{\rm E} \leq b_{\rm w} + 6t$
- 3) $b_{\rm E} \le b_{\rm w} + b_{\rm 0}$

Isolated T-section:

- $1)\ b_{\rm E} \leq 4\ b_{\rm w}$
- 2) $t \ge b_w/2$





Width of Flange

Width of Flange, b:

When extension in both sides (Interior T Section)

$$\blacktriangleright$$
 b $\leq \frac{L}{4}$

$$\triangleright$$
 b \leq 16 $t + b'$

$$\triangleright$$
 b $\leq l + b'$

When extension in one side(Exterior T Section)

$$\blacktriangleright$$
 b $\leq \frac{L}{12}$

$$\triangleright$$
 b \leq 6t

$$\blacktriangleright$$
 b $\leq \frac{l}{2}$

For excess compressive area of flange (Isolated)

$$\succ t \ge \frac{b'}{2}$$
 and

$$> b \leq 4b'$$

Selection of b_E

Interior T-section:

1)
$$b_{\rm E} \le L/4$$

2)
$$b_{\rm E} \le b_{\rm w} + 16t$$

3)
$$b_{\rm E} \le \text{c-c}$$
 of beams

Exterior T-section:

1)
$$b_{\rm E} \le b_{\rm w} + L / 12$$

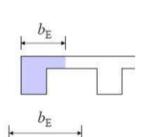
2)
$$b_{\rm E} \le b_{\rm w} + 6t$$

3)
$$b_{\rm E} \le b_{\rm w} + b_0$$

Isolated T-section:

1)
$$b_{\rm E} \leq 4 b_{\rm w}$$

2)
$$t \ge b_w/2$$



Step-1: Load Calculation

Given data -

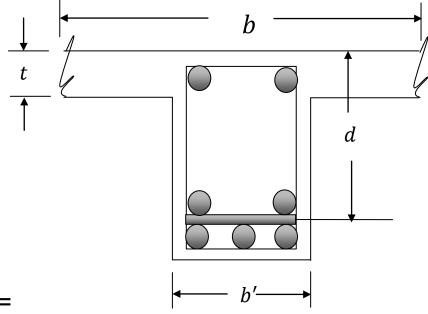
$$f_C'/f_C =$$

$$f_{S} =$$

$$n =$$

L=

Super imposed load (L.L+ Others Loads except D.L) =



[Note: Self weight of web is Dead load here & D.L= 10%-15% of super imposed load]

Step-1: Load Calculation

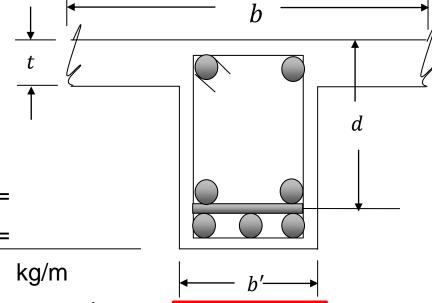
Let

$$d = 10 \% \text{ of L}$$

$$b' = \frac{1}{3} to \frac{1}{2}$$
 of d

Super imposed load (L.L+ Wt. of slab + Others Loads) =

Self weight of web (D.L) = 10 % of Super imposed load



$$\omega = \text{kg/m}$$

or
$$W = kg$$

[Super imposed load = L.L+ other loads + wt. of slab + wt. of lime concrete + etc.]

Step-2: $V_{\text{max}} = Maximum Shear force$

$$V = \frac{W}{2} = \frac{\omega L}{2}$$
 for simply supported Beam

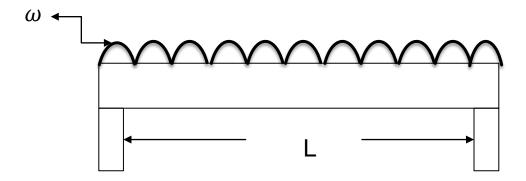
$$V = \frac{W}{2} = \frac{\omega L}{2}$$
 for fully Continuous Beam

V = 0.6
$$W = .6\omega L$$
 for semi- continuous Beam

(Continuous end)

$$V = 0.4 W = .4\omega L$$
 for semi- continuous Beam

(Dis-continuous end)



$$V = R = \frac{W}{2}$$

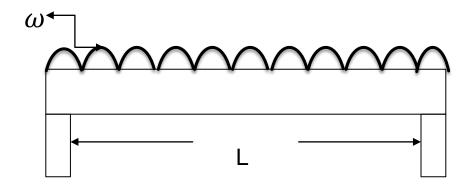
Step-3: $M_{\text{max}} = Maximum Bending Moment$

$$M = \frac{WL}{8} = \frac{\omega L^2}{8}$$
 kg.m for simply supported Beam

$$M = \frac{WL}{12} = \frac{\omega L^2}{12}$$
 kg.m for fully Continuous Beam

$$M = \frac{WL}{10} = \frac{\omega L^2}{10}$$
 kg.m for semi- continuous Beam

$$M = \frac{WL}{2} = \frac{\omega L^2}{2}$$
 kg.m for cantilever Beam



To covert those moments in kg.cm we have to multiply by 100

i.e. the formulas will like this $\mathbf{M} = \frac{\omega L^2}{8} \times 100$ kg.cm. It will be apply for all beams.

Dear students, have any question?

if any questions then comment here & contact with me!

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If You are not willing to LEARN no one can HELP you.
If You are DETERMINED to learn no one can STOP You!

Welcome to next class!

Thank You

Practice in your home



Topic

Teacher Info
Subect

Width of Flange

Width of Flange, b:

When extension in both sides

$$\triangleright$$
 b $\leq \frac{L}{4}$

$$\triangleright$$
 b \leq 16 $t + b'$

$$\triangleright$$
 b $\leq l + b'$

When extension in one side

$$\rightarrow$$
 b $\leq \frac{L}{12}$

$$\triangleright$$
 b \leq 6t

$$\blacktriangleright$$
 b $\leq \frac{l}{2}$

For excess compressive area of flange

$$rac{b'}{2}$$
 and

$$> b \le 4b'$$

Selection of b_E

Interior T-section:

1)
$$b_{\rm E} \le L/4$$

2)
$$b_{\rm E} \le b_{\rm w} + 16t$$

3)
$$b_{\rm E} \le \text{c-c}$$
 of beams

Exterior T-section:

1)
$$b_{\rm E} \le b_{\rm w} + L / 12$$

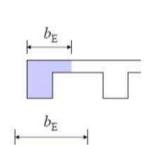
2)
$$b_{\rm E} \le b_{\rm w} + 6t$$

3)
$$b_{\rm E} \le b_{\rm w} + b_0$$

Isolated T-section:

1)
$$b_{\rm E} \leq 4 b_{\rm w}$$

2)
$$t \ge b_w/2$$



Step -4: Effective depth, d

$$b'd = \frac{V}{v}$$
 (Original formula, $v = \frac{V}{b'd}$)

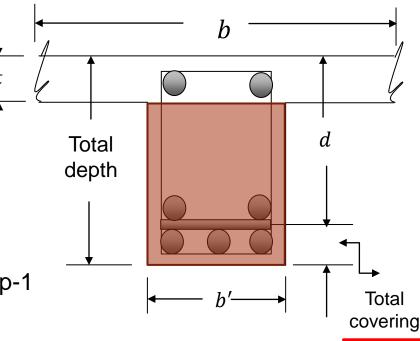
 $v = \text{Maximum shear stress of concrete} = 1 \cdot 33\sqrt{f_c'}$

After calculating *d* the size of beam will be fixed then

Check of wt. of web:

the load of wed should be check by the equation

= $L \times b' \times (\text{Total depth } - t) \times 2400 < \text{assumed weight of web in step-1}$



d = Total depth - total covering

Total covering = $5 + \phi + \frac{\phi}{2}$ [You can take Total covering 7.5 cm]

Step – 5 : Area of Steel, A_s

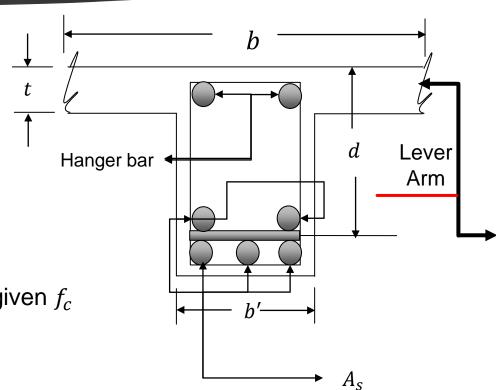
$$A_S = \frac{M}{f_S\left(d - \frac{t}{2}\right)} \quad cm^2$$

Here, Lever arm = $d - \frac{t}{2}$

Find the total number of bars by using area of bars.

Then check f_c

by using this formula $f_c = \frac{2M}{b \times t \left(d - \frac{t}{2}\right)}$ and this should be less than given f_c



Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{b'd} \qquad [V_{cr} = V - \frac{\omega d}{100}]$$

here, $V_{cr} = V_d = V$ at d distance from support.

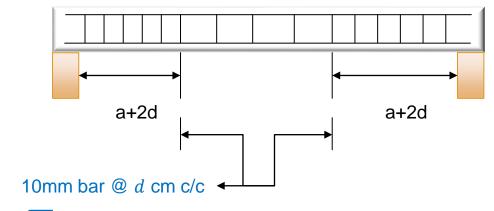
[100 is to convert d in meter]

[This is actual Shear stress ($v = v_{ac}$)]

But allowable stress of concrete is, $v_C = 0 \cdot 292\sqrt{f_C'}$

If $v > v_c$ stirrups required.

and access shear stress, $v' = v - v_c$



[Maximum shear stress of Beam = $1 \cdot 33\sqrt{f_C'}$, if $v < 1 \cdot 33\sqrt{f_C'}$ it will be safe in shear]

[Note: if $v < v_c$ then stirrups should be given at the spacing of d]

Step -7: Space required for stirrups, a

```
a=\left(rac{L}{2}-d
ight)rac{v'}{v} for simply supported Beam a=\left(rac{L}{2}-d
ight)rac{v'}{v} for fully continuous Beam a=(0.6L-d)rac{v'}{v} for semi - continuous Beam (at continuous end) a=(0.4L-d)rac{v'}{v} for semi - continuous Beam (at discontinuous end) a=(L-d)rac{v'}{v} for cantilever Beam [Here access shear stress v'=v-v_c]
```

Total space required for stirrups = a + 2d [according to ACI]

[Note: Value of "L" should be in cm.]

Step-8: Spacing of Stirrups, S

$$S = \frac{A_v f_v}{v'b'}$$

$$S = \frac{A_v}{0.0015b'}$$

3)
$$s = \frac{d}{2}$$

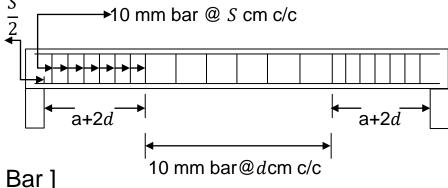
Here, $A_v = \text{Area of stirrups} = \text{Area of one stirrup x leg(s)}$

[normally stirrups are 10 mm dia. Bar]

 f_v = Allowable stress of stirrups

The minimum will be the Spacing of Stirrups.

[Note: Usually $f_v = f_s$]



Step-9: Check for Bond stress, u

$$u = \frac{V}{\sum 0(d - \frac{t}{2})}$$
 Actual bond stress

Here, $\sum 0 = N\pi D$, D = Dia. Of main bars, N = Number of main bar

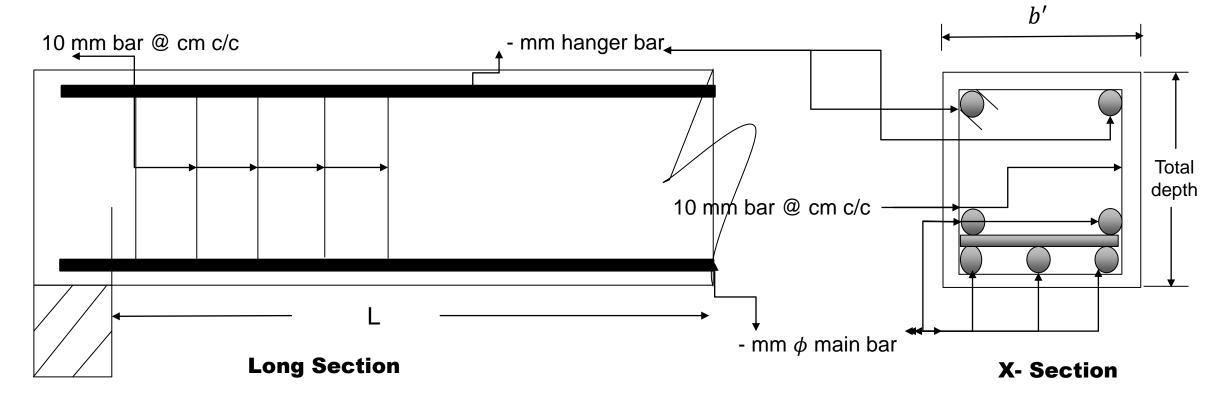
$$u_{all} = \frac{3 \cdot 23 \sqrt{f_C'}}{D}$$
 Allowable bond stress

If $u < u_{all}$

safe in bond.

Home work: 1) Memorize all formulas from step-1 to step-9, 2) understand all steps of rectangular beam design (WSD Method) and try to design a beam from a example in your books! Submit to you Teacher after opening the institutions!

Step-10: Detail drawing



Dear students, have any question?

if any questions then comment here & contact with me!

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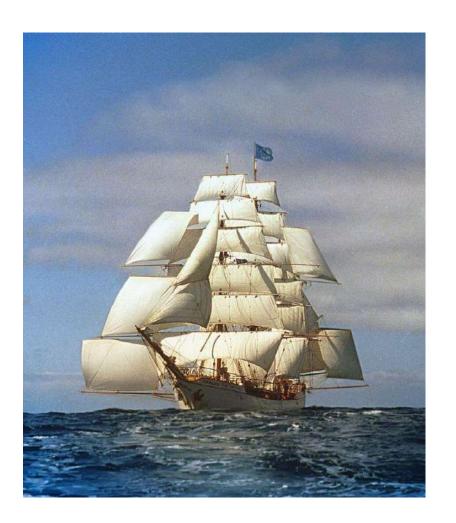
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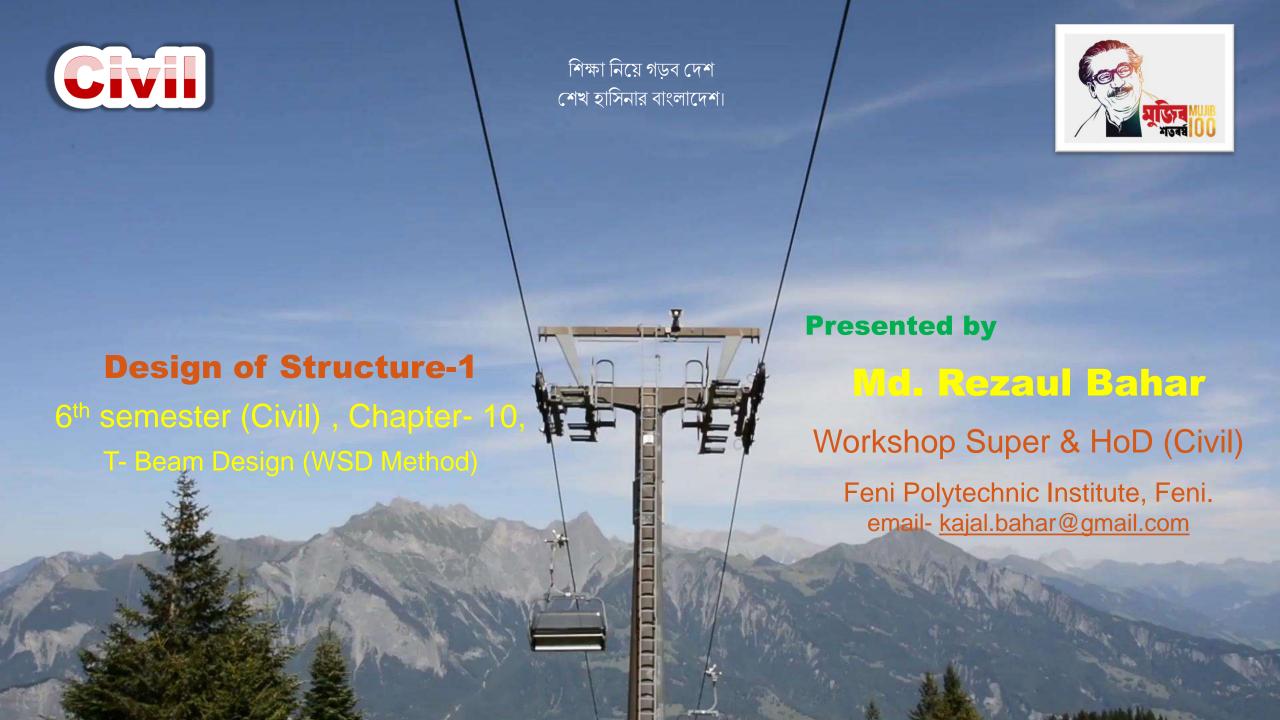
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Start working hard with right policies and get SUCCESS!

Thank You





3 STEPS YOU SHOULD BE ALERT



একটি সাধারণভাবে স্থাপিত টী-বিমের ন্প্যান 5 মিটার l স্লাবের পুরুত্ব 15 সেমি, জলছাদের পুরত্ব 7.5 সেমি, লাইভ লোড 500 কেজি/বর্গ মিটার, বিম স্পেশিং 4 মিটার হলে নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর l

$$f_c = 95 \; k \; g/c \; m^2$$
 , $f_s = 1400 \; k \; g/c \; m^2$, $n = 9$, $u = 16 \; k \; g/c \; m^2$, $v = 18 \; k \; g/c \; m^2$, $v_c = 4.24 \; k \; g/c \; m^2$

Step-1: Load Calculation

```
L.L = 5 \times 4 \times 500 = 10000 kg
```

Weight of slab =
$$5 \times 4 \times 0.15 \times 2400 = 7200 \text{ kg}$$

Weight of L.C =
$$5 \times 4 \times 0.075 \times 1920 = 2880 \text{ kg}$$

Weight of web (15%)
$$=$$
 3012 kg

Total weight,
$$W = 23092 \text{ kg}$$

Step-2: $V_{\text{max}} = Maximum Shear force$

$$V = \frac{W}{2} = \frac{23092}{2} = 11546 \text{ kg}$$

Step-3: $M_{\text{max}} = Maximum Bending Moment$

$$M = \frac{WL}{8} \times 100 = \frac{23092 \times 5}{8} \times 100 = \underline{1443250 \text{ kg.cm}}$$

Step -4: Effective depth, d

$$b'd = \frac{v}{v}$$

Let,
$$b' = 20 \text{ cm}$$

$$d = \frac{V}{v \times b'} = \frac{11546}{18 \times 20} = 32.07 \text{ cm}$$

Total depth =
$$32.07 + 5 + 2.5 + \frac{2.5}{2} = 41.82 = 200 + 42 cm$$

Hence, Size of T-Beam 20 cm x 42 cm.

$$d = 42 - 5 - 2.5 - 1.25 = 33.25$$
 cm

Check of weight of web =
$$5 \times \frac{20}{100} \times \frac{42-15}{100} \times 2400 = 648 \text{ kg} < 3012 \text{ kg}$$
, safe.

Step – 5 : Area of Steel, A_s

$$A_S = \frac{M}{f_S(d - \frac{t}{2})} = \frac{1443250}{1400(33 \cdot 25 - \frac{15}{2})} = 40.03 \ cm^2$$

by using 5 nos. 32 mm ϕ bar the area is 40.19 $cm^2 > 40.03$ cm^2

Use $5 - 32 \text{ mm } \phi$ bar as main bar.

Check of f_c

1)
$$b = \frac{L}{4} = \frac{500}{4} = 125 \text{ cm}$$
, 2) $b = 16t + b' = 16 \times 15 \times 20 = 260 \text{ cm}$, 3) $b = l = 400 \text{ cm}$

The minimum is 125 cm, hence b = 125 cm

$$f_c = \frac{2M}{b \times t \left(d - \frac{t}{2}\right)} = \frac{2 \times 1443250}{125 \times 15 \left(33 \cdot 25 - \frac{15}{2}\right)} = \underline{59.78 \ kg/cm^2 < 95 \ kg/cm^2}$$
, safe.

Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{b'd} = \frac{10011}{20 \times 33.75} = 15.05 kg/cm^2 < 16 kg/cm^2$$
, hence safe.

$$V_{cr} = V - \frac{Wd}{L \times 100} = 11546 - \frac{23092 \times 33 \cdot 25}{5 \times 100} = 10011 \text{ kg}$$

But allowable stress of concrete is, $v_C = 4.24 \ kg/cm^2$

If $v > v_c$ stirrups required.

and access shear stress, $v' = v - v_c = 15.05$ - 4.24 = 10.81 kg/cm^2

Step -7: Space required for stirrups, a

$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v} = \left(\frac{500}{2} - 33 \cdot 25\right) \frac{10.81}{15.05} = 156 \text{ cm}$$

Total space required for stirrups = a + 2d = 156 + 2x 33.25 = 223 cm = 2.23 m

Step-8: Spacing of Stirrups, S

1)
$$S = \frac{A_v f_v}{v'b'} = \frac{1.58 \times 1400}{10.81 \times 20} = 10.23 \text{ cm}$$

2)
$$S = \frac{A_v}{0.0015b'} = \frac{1.58}{0.0015 \times 20} = 53 \text{ cm}$$

3)
$$S = \frac{d}{2} = \frac{33 \cdot 25}{2} = 16.5 \text{ cm}$$

S = 10 cm

Use 10 mm ϕ bar @ 10 cm c/c as stirrups

[
$$A_v$$
= 0.79 x 2 = 1.58 cm^2 , $f_v = f_s = 1400$

Step-9: Check for Bond stress, u

$$u = \frac{V}{\sum 0(d - \frac{t}{2})} = \frac{11546}{50.24(33.25 - \frac{15}{2})} = 8.92 \, kg/cm^2$$

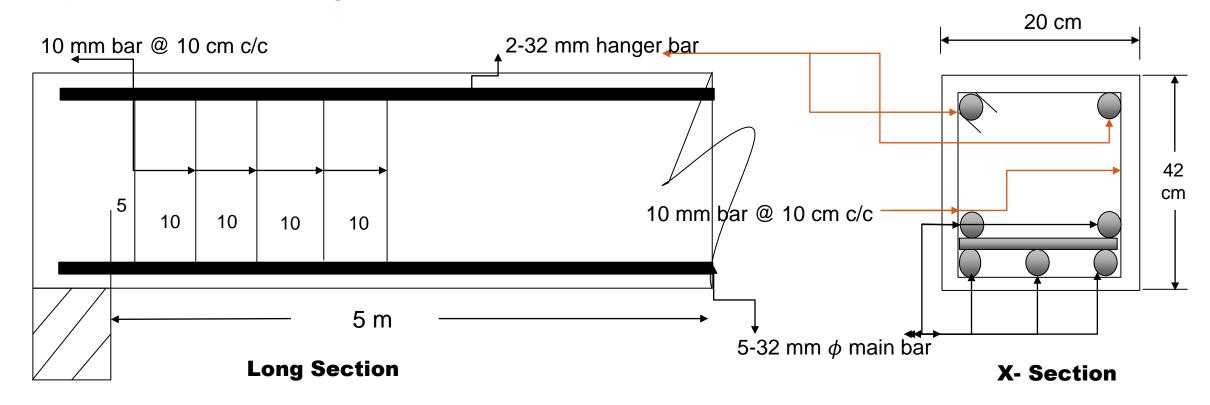
Here, $\sum 0 = N\pi D = 5 \times 3.14 \times 3.2 = 50.24 \text{ cm}$

$$u_{all} = 18 k g/c m^2$$

If $u < u_{all}$

safe in bond.

Step-10: Detail drawing



- 1) একটি সাধারণভাবে স্থাপিত টী-বিমের ন্প্যান 6 মিটার l স্লাবের পুরুত্ব 14 সেমি, লাইভ লোড 500 কেজি/বর্গ মিটার, বিম ম্পেশিং 4 মিটার হলে নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর l
 - $f_c = 90 \; k \; g/c \; m^2$, $f_s = 1400 \; k \; g/c \; m^2$, n = 10 , $u = 16 \; k \; g/c \; m^2$, $v = 18 \; k \; g/c \; m^2$, $v_c = 4.20 \; k \; g/c \; m^2$
- 2) একটি সম্পূর্ণ অবিছিন্ন টী-বিমের ন্প্যান 5 মিটার l স্লাবের পুরুত্ব 15 সেমি, জলছাদের পুরুত্ব 7.5 সেমি, লাইভ লোড 550 কেজি/বর্গ মিটার, বিম ম্পেশিং 3.5 মিটার হলে নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর l
 - $f_c = 94.5 \ k \ g/c \ m^2$, $f_s = 1450 \ k \ g/c \ m^2$, n = 9 , $u = 16 \ k \ g/c \ m^2$, $v = 18 \ k \ g/c \ m^2$, $v_c = 4.23 \ k \ g/c \ m^2$
- 3) একটি আংশিক অবিছিন্ন টী-বিমের ন্প্যান 5.5 মিটার l স্লাবের পুরুত্ব 14 সেমি, লাইভ লোড 600 কেজি/বর্গ মিটার, বিম ম্পেশিং 4 মিটার হলে নিম্নের তথ্যাদির সাহায্যে বিমটি ডিজাইন কর l
 - $f_c = 95 \; k \; g/c \; m^2$, $f_s = 1450 \; k \; g/c \; m^2$, n = 10 , $u = 16 \; k \; g/c \; m^2$, $v = 18 \; k \; g/c \; m^2$, $v_c = 4.23 \; k \; g/c \; m^2$

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Practice in your house & do the home works. welcome to next classes!

Thank You

Practice a lot at home!



WELCOME TO MY PRESENTATION





Double Reinforced Beam

- 1. Concept of Double reinforced Beam
- 2. Load Calculation
- 3. Moment Calculation
- 4. Area of Tensile Reinforcement
- 5. Area of Compression Reinforcement

2

3

4

Concept of Double Reinforced Beam



When the size of the beam (both width and depth) is restricted.



When is moment of resistance required is too high that it results in very large beam.



When there is reversal of load in beams.



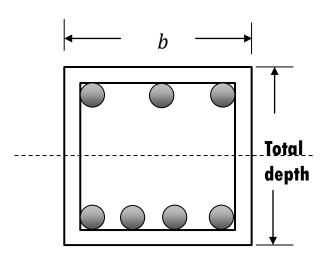
The deflection due to shrinkage effects.

Load Calculation

Step-1: Load Calculation

$$\omega$$
 = kg/m
or W = kg

[Note: if Super imposed load given in kg/m then L = 1 m]





Maximum Shear Force

Step-2: $V_{\text{max}} = Maximum Shear force$

for cantilever Beam

$$V = \frac{W}{2} = \frac{\omega L}{2}$$
 for simply supported Beam $V = \frac{W}{2} = \frac{\omega L}{2}$ for fully Continuous Beam $V = 0.6 \ W = .6 \omega L$ for semi- continuous Beam (Continuous end) $V = 0.4 \ W = .4 \omega L$ for semi- continuous Beam (Dis-continuous end)

V = W

Moment Calculations

Step-3: $M_{\text{max}} = Maximum Bending Moment$ **Bending Moment**

$$M = \frac{WL}{8} = \frac{\omega L^2}{8}$$
 kg.m for simply supported Beam

$$M = \frac{WL}{12} = \frac{\omega L^2}{12}$$
 kg.m for fully Continuous Beam $M = \frac{WL}{10} = \frac{\omega L^2}{10}$ kg.m for semi- continuous Beam $M = \frac{WL}{2} = \frac{\omega L^2}{2}$ kg.m for cantilever Beam

$$M = \frac{WL}{10} = \frac{\omega L^2}{10}$$
 kg.m for semi- continuous Beam

$$M = \frac{WL}{2} = \frac{\omega L^2}{2}$$
 kg.m for cantilever Beam

Resisting Moment

$$M_1 = Rbd^2$$

Access Moment

$$M_2 = M - M_1$$



Area of Tensile Reinforcements

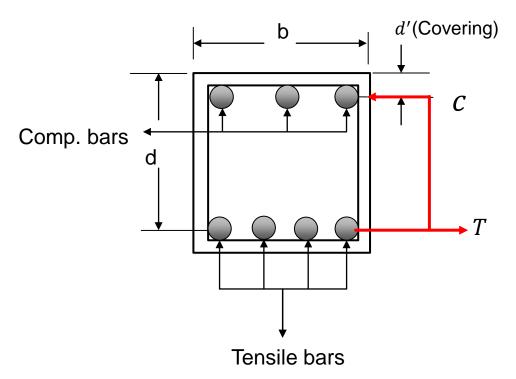
Tensile reinforcement

$$A_{S_1} = \frac{M_1}{f_S j d} =$$
 $A_{S_2} = \frac{M_2}{f_S (d - d')}$

Total Tensile reinforcement

$$A_{\scriptscriptstyle S} = A_{\scriptscriptstyle S_1} + A_{\scriptscriptstyle S_2}$$

Total number of rebars should be calculate with dia. as usual.





Area of Compression Reinforcements

Compressive reinforcement

$$A_S' = \frac{C_2}{2nf_C(kd-d')/kd} =$$

Here,

$$M_2 = C_2(d - d') =$$

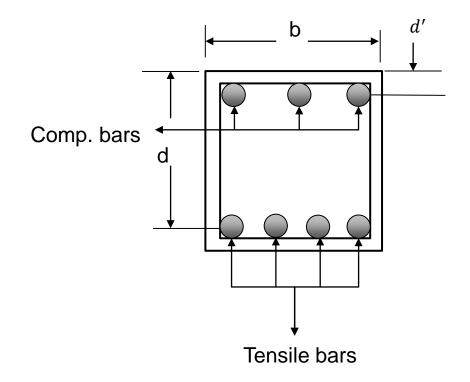
Hence
$$c_2 = \frac{M_2}{d-d'}$$

Total number of rebars should be calculate with dia. as usual.

Check of Compressive stress

Compressive stress of steel =
$$\frac{2nf_c(k \, d-d')}{kd}$$

And
$$\frac{2nf_c(k d-d')}{kd} < f_s$$







Check for Shear Stress

Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{bd} \qquad [V_{cr} = V - \frac{\omega d}{100}]$$

But allowable stress of concrete is, $v_C = 0 \cdot 292\sqrt{f_C'}$ If $v > v_c$ stirrups required. and access shear stress, $v' = v - v_c$

[Maximum shear stress of Beam = $1 \cdot 33\sqrt{f_C'}$, if $v < 1 \cdot 33\sqrt{f_C'}$ it will be safe in shear] [Note: if $v < v_c$ then stirrups should be given at the spacing of d according to ACI code]

Check for Shear Stress

Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{bd} \qquad [V_{cr} = V - \frac{\omega d}{100}]$$

here, $V_{cr} = V_d = V$ at d distance from support.

[100 is to convert d in meter]

[This is actual Shear stress ($v = v_{ac}$)]

But allowable stress of concrete is, $v_C = 0 \cdot 292\sqrt{f_C'}$

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Stirrups Space

Step -7: Space required for stirrups, a

$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v}$$
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[Here access shear stress $v' = v - v_c$]

Total space required for stirrups = a + 2d [according to ACI] [Note: Value of "L" should be in cm. because d is in cm.]

Spacing of Stirrups

Step-8: Spacing of Stirrups, S

$$1)S = \frac{A_v f_v}{v'b} \ ,$$

$$2)S = \frac{A_v}{0.0015b} \ ,$$

$$3)s = \frac{d}{2}$$
Here, $A_v = \text{Area of stirrups} = \text{Area of one stirrup x leg(s)}$

$$[\text{normally stirrups are 10 mm dia. Bar }]$$

$$f_v = \text{Allowable stress of stirrups}$$
The minimum will be the Spacing of Stirrups.
$$[\text{Note: Usually } f_v = f_s]$$

Spacing of Stirrups

Step-9: Check for Bond stress, u

$$u = \frac{V}{\sum 0 jd}$$
 Actual bond stress

Here, $\sum 0 = N\pi D$, D = Dia. Of main bars, N = Number of main bar

$$u_{all} = \frac{3 \cdot 23 \sqrt{f_C'}}{D}$$
 Allowable bond stress

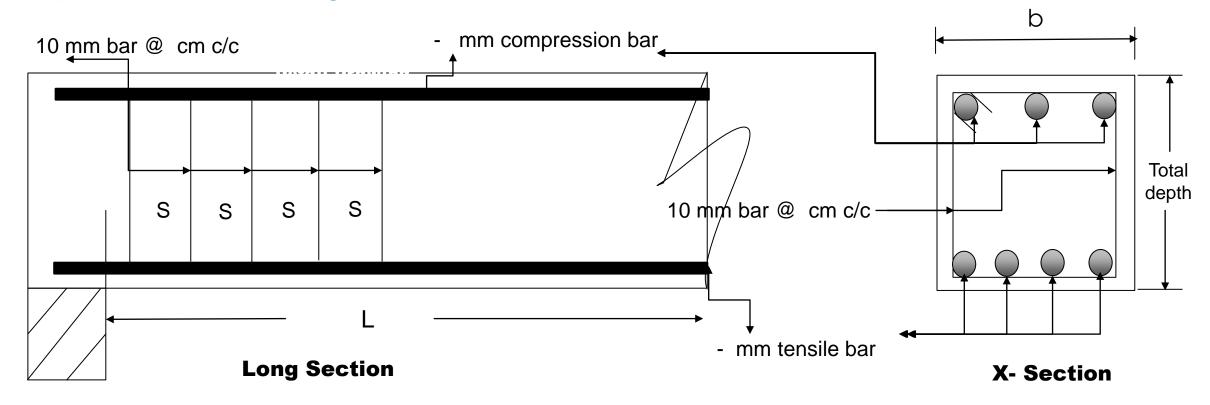
If $u < u_{all}$

safe in bond.

Home work: 1) Memorize all formulas from step-1 to step-9, 2) understand all steps of rectangular beam design (WSD Method) and try to design a beam from a example in your books! Submit to you Teacher after opening the institutions!

Detail Drawing

Step-10: Detail drawing



Dear students, have any question?

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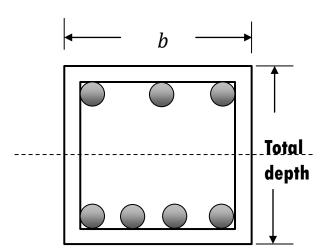
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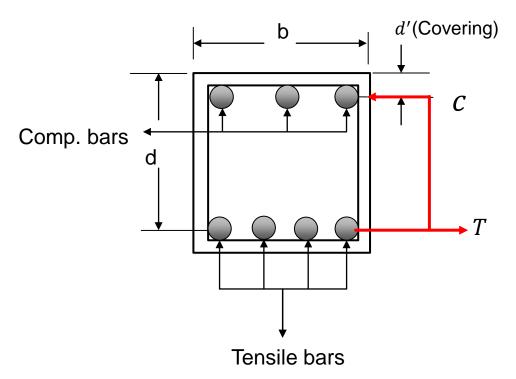
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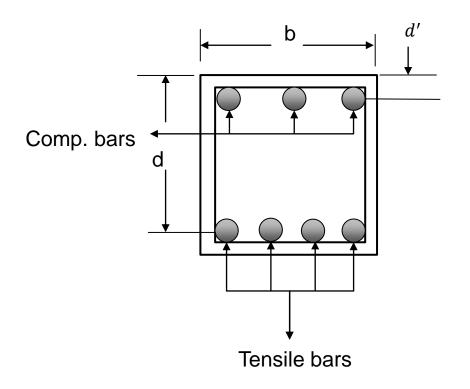
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[Here access shear stress $v' = v - v_c$]

Total space required for stirrups = a + 2d [according to ACI] [Note: Value of "L" should be in cm. because d is in cm.]

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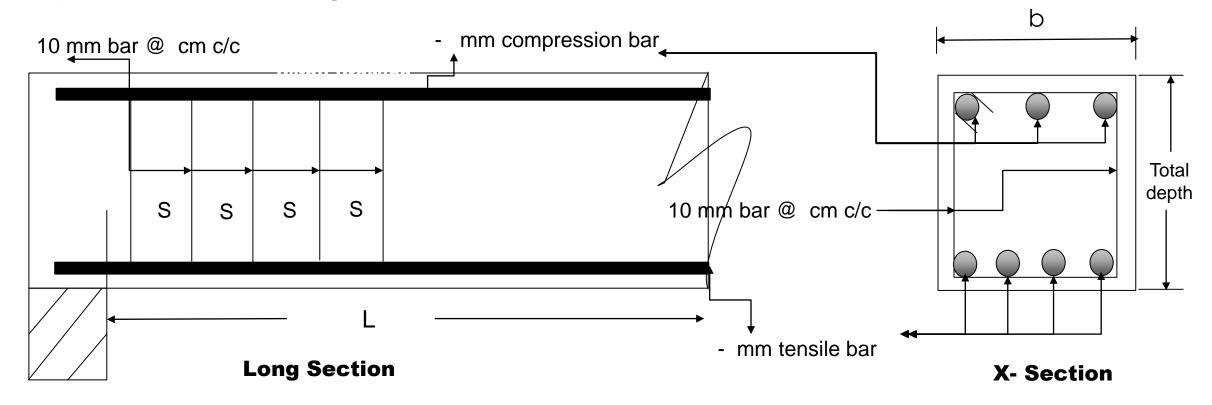
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Thank You

Make yourself a qualified one!





Diploma in Engineering Civil Technology

Md. Anwar Hossain, Roll no: 532020, Reg. no: 664422

CGPA-3.91,

Design of a Double Reinforced Beam

একটি সাধারণ ভাবে স্থাপিত আয়াতাকার বিমের স্প্যান 5 মিটার যাহার আকার 25 সেমি. x 35 সেমি. এ সীমিত | উহার উপর নিজস্ব ওজন ছাড়া মোট 14000 কেজি লোড আরোপিত আছে | নিম্নের তথ্যাদির সাহায্যে বিমটির main bar ও stirrups ডিজাইন কর তথ্যাদি-

 $f_c' = 210 \ k \ g/c \ m^2$, $f_s = 1450 \ k \ g/c \ m^2$, n = 10, Total covering = 5 cm, $v_c = 4.23 \ k \ g/c \ m^2$.

- 1) Load cal.
- 3) Tensile Rebars
- 5) Stirrups Design
- 2) Moment cal.
- 4) Comp. Rebars

Load Calculation

Step-1: Load Calculation

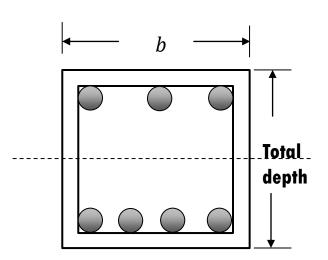
Super imposed load (L.L+ Others Loads)

= 14000 kg

Self weight of Beam (D.L) = $5 \times 0.25 \times 0.35 \times 2400$

= 1050 kg

W = 15050 kg





Maximum Shear Force

Step-2: $V_{\text{max}} = Maximum Shear force$

$$V = \frac{W}{2} = \frac{15050}{2} = 7525 \text{ kg}$$

Moment Calculations

Step-3: $M_{\text{max}} = Maximum \ Bending \ Moment$

Bending Moment

$$M = \frac{WL}{8} = \frac{15050 \times 5}{8} = 9406.25 \text{ kg.m} = 940625 \text{ kg.cm}$$

Resisting Moment

$$M_1$$
= Rbd^2 = 16 x 25 x 30^2 = 360000 kg.cm

[Here, 0.45
$$f'_c = 0.45 \times 210 = 94.5 \ k \ g/c \ m^2$$
, $d = 35 - 5 = 30$
 $k = \frac{n}{n + \frac{f_s}{f_c}} = \frac{10}{10 + \frac{1450}{94}} = 0.39$, $j = 1 - \frac{k}{3} = 1 - \frac{0.46}{3} = 0.87$, $R = \frac{1}{2} f_c j k = 16$]

Access Moment

$$M_2 = M - M_1 = 940625 - 360000 = 580625 \text{ kg.cm}$$

Area of Tensile Reinforcements

Tensile reinforcement

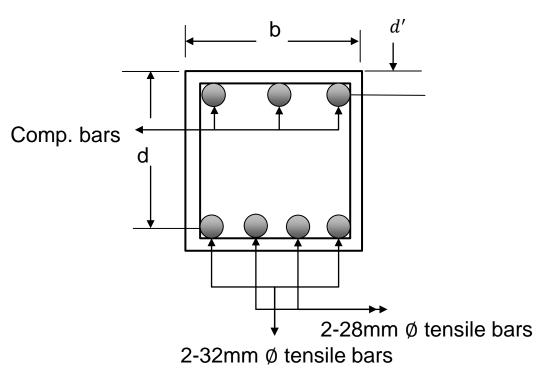
$$A_{S_1} = \frac{M_1}{f_S j d} = \frac{360000}{1450 \times 0.87 \times 30} = 9.51 \ cm^2$$

$$A_{S_2} = \frac{M_2}{f_S(d-d')} = \frac{580625}{1450 \times 25} = \underline{16.02 \ cm^2}$$

Total Tensile reinforcement

$$A_S = A_{S_1} + A_{S_2} = 9.51 + 16.02 = 25.53 \ cm^2$$

By using 2 - 32 mm \emptyset + 2 - 28 \emptyset bar the area is = 28.38 cm^2 > 25.53 cm^2 Use 2 - 32 mm \emptyset and 2 - 28 mm \emptyset bar as main bar.





Area of Compression Reinforcements

Compressive reinforcement:

$$A'_{S} = \frac{C_{2}}{2nf_{C}(kd-d')/kd} = \frac{23225}{2\times10\times94.5(0.39\times25-5)/0\cdot39\times25} = \frac{25.23 \ cm^{2}}{2}$$

Here,

$$c_2 = \frac{M_2}{d-d'} = \frac{580625}{25} = 23225 \text{ kg} \left[d - d' = 30 - 5 = 25 \right]$$
 Ø Comp. bars d

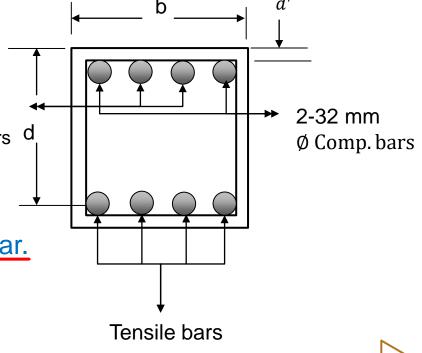
By using $2 - 32 \text{ mm } \emptyset + 2 - 25 \emptyset$ bar the area is

 $= 25.89 \ cm^2 > 25.23 \ cm^2$

Use 2 - 32 mm Ø and 2 - 25 mm Ø bar as main compression bar.

Check:

Compressive stress of steel = $\frac{2nf_c(k d-d')}{kd}$ = $\frac{921 \ k \ g/c \ m^2}{1450 \ k \ g/c \ m^2}$, hence safe.



2-25 mm

Check for Shear Stress

Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{bd} = \frac{6622}{25 \times 30} = 8.83 \quad k \, g/c \, m^2$$

Here,
$$V_{cr} = V - \frac{\omega d}{100} = 7525 - \frac{3010 \times 30}{100} = 6622 \text{ kg}$$

But allowable stress of concrete is, $v_C = 4.23 \ k \ g/c \ m^2$

 $v > v_c$ stirrups required.

and access shear stress, $v' = v - v_c = 8.83 - 4.23 = 4.60 \text{ kg/cm}^2$

[Maximum shear stress of Beam = $1 \cdot 33\sqrt{f_C'}$, if $v < 1 \cdot 33\sqrt{f_C'}$ it will be safe in shear]

Stirrups Space & Spacing

Step -7: Space required for stirrups, a

$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v} = \left(\frac{500}{2} - 30\right) \frac{4.60}{8.83} = 115 \text{ cm}$$

Total space required for stirrups = $a + 2d = 115 + 2 \times 30 = 175$ cm

Step-8: Spacing of Stirrups, S

1)
$$S = \frac{A_v f_v}{v'b} = \frac{1.58 \times 1450}{4.60 \times 25} = 19.92 \text{ cm}$$
 Here, $A_v = \frac{\pi \times 1.0^2}{4} \times 2 = 1.58$ [Ring stirrups has 2 leg], $f_v = f_s$

2)
$$S = \frac{A_v}{0.0015b} = \frac{1.58}{0.0015 \times 25} = 42.13 \text{ cm}$$

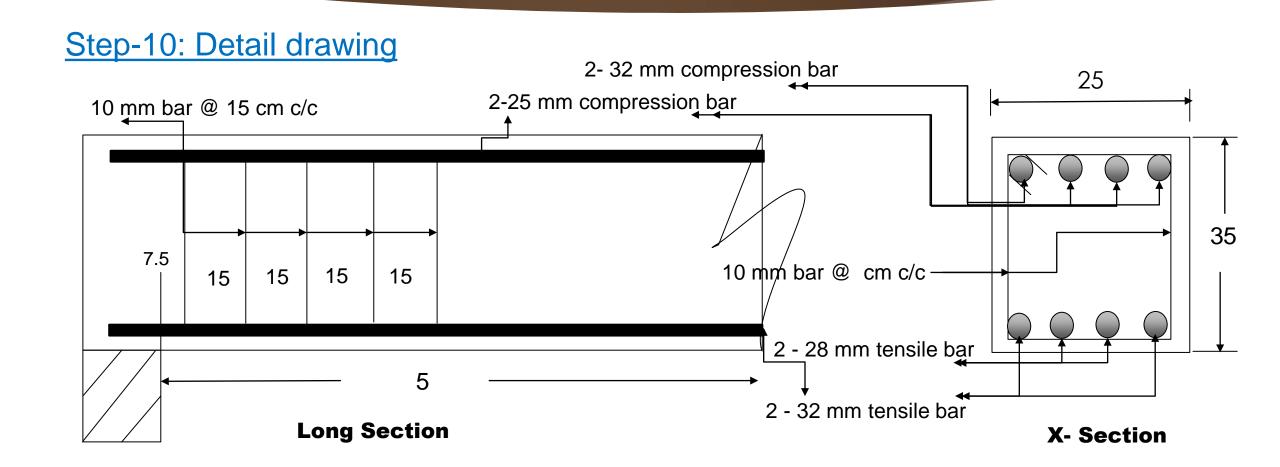
3)
$$s = \frac{d}{2} = \frac{30}{2} = 15 \text{ cm}$$

The minimum spacing is 15 cm

Use 10 mm Ø bar @ 15 cm c/c



Detail Drawing



Dear students, have any question?

if any questions then comment here & contact with me!

আর -

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Practice in your house & do the home works. welcome to next classes!

Thank You

Practice a lot at home!



শিক্ষা নিয়ে গড়ব দেশ শেখ হাসিনার বাংলাদেশ।







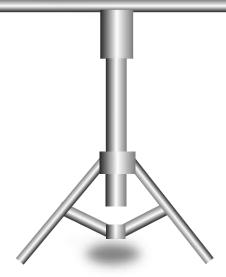
Design of Structure-1

6th semester (Civil),Chapter- 07, Lintel Design (WSD Method)

Presented by: Md. Rezaul Bahar

Workshop Super & HoD (Civil)

Feni Polytechnic Institute, Feni. email- kajal.bahar@gmail.com













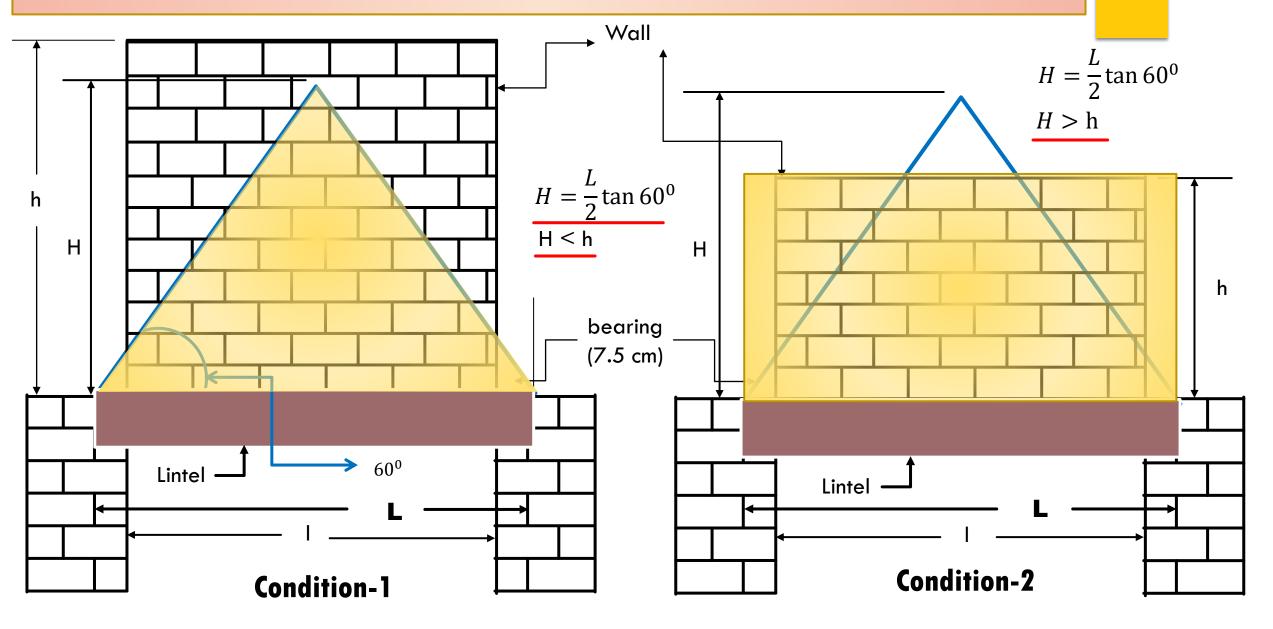








Lintel under Load



Problem to Solve



Step-1: Load Calculation

Given data -

$$f_C'/f_C = ,f_S = ,n =$$

| =

L = I + bearing (15 cm)

Weight of wall:

- 1. If condition-1 the wt. of wall = $L \times L \tan 60^{\circ} \times 10^{\circ}$ width of wall $\times 1920^{\circ}$
- 2. If condition-2 the wt. of wall = L x height of wall x width of wall x 1920

Step-1: Load Calculation

```
Bearing for Lintel
```

7.5 cm for both sides (i.e. half of the lintel height usually),

Hence, L = I + 15 cm

Let,

Depth of Lintel = 15 cm (for long span or heavy structure maybe up to 20 cm)

Width of Lintel = Width of wall (25 cm)

Weight of wall (W_1) = L x width of wall x height of wall x 1920 =

Self weight of Lintel (W_2) = L x 0.25 x 0.15 x 2400 =

W = kg

Step-2: $V_{\text{max}} = Maximum Shear force$

$$V = \frac{W}{2}$$

Step-3: $M_{\text{max}} = Maximum Bending Moment$

For the weight of wall, $M_1 = \frac{W_1L}{6}$

For the weight of Lintel, $M_2 = \frac{W_2L}{8}$

Total moment, $M = M_1 + M_2$

Step-4:
$$d = Effective depth$$

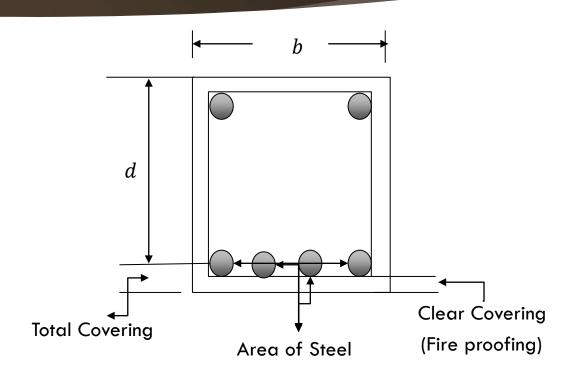
$$d = \sqrt{\frac{M}{Rb}}$$

$$R = \frac{1}{2} f_c j k \qquad [k = \frac{n}{n + \frac{f_s}{f_c}} \text{ and } j = 1 - \frac{k}{3}]$$

Total depth = d + total covering

Total covering =
$$3 + \frac{\theta}{2}$$
 $\theta = Dia \ of \ main \ bar$ => $d = 15 - 3 - \frac{\theta}{2}$ [here Clear covering = 3]

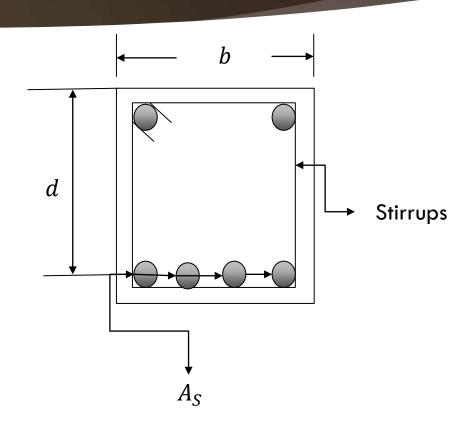
$$\Rightarrow d = 15 - 3 - \frac{\theta}{2}$$
 [here Clear covering = 3]



Step-5: A_s = Area of Steel

$$A_S = \frac{M}{f_S j d} \qquad cm^2$$

Find the total number of bars By using area of bars.



Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{bd} \qquad [V_{cr} = V - \frac{\omega d}{100}]$$

here, $V_{cr} = V_d = V$ at d distance from support.

allowable stress of concrete is, $v_C = 0 \cdot 292 \sqrt{f_C'}$ If $v > v_c$ stirrups required.

and access shear stress, $v' = v - v_c$

Step -7: Space required for stirrups, a

$$a = \left(\frac{L}{2} - d\right) \frac{v'}{v}$$
 for simply supported Beam

Total space required for stirrups = a + 2d [according to ACI]

Step-8: Spacing of Stirrups, S

$$1) \quad S = \frac{A_v f_v}{v'b} \quad ,$$

2)
$$S = \frac{A_v}{0.0015b}$$
,

3)
$$s = \frac{d}{2}$$

The minimum will be the Spacing of Stirrups

Step-9: Check for Bond stress, u

$$u = \frac{V}{\sum 0 \, id}$$
 Actual bond stress

Here, $\sum 0 = N\pi D$, D = Dia. Of main bars, N = Number of main bar

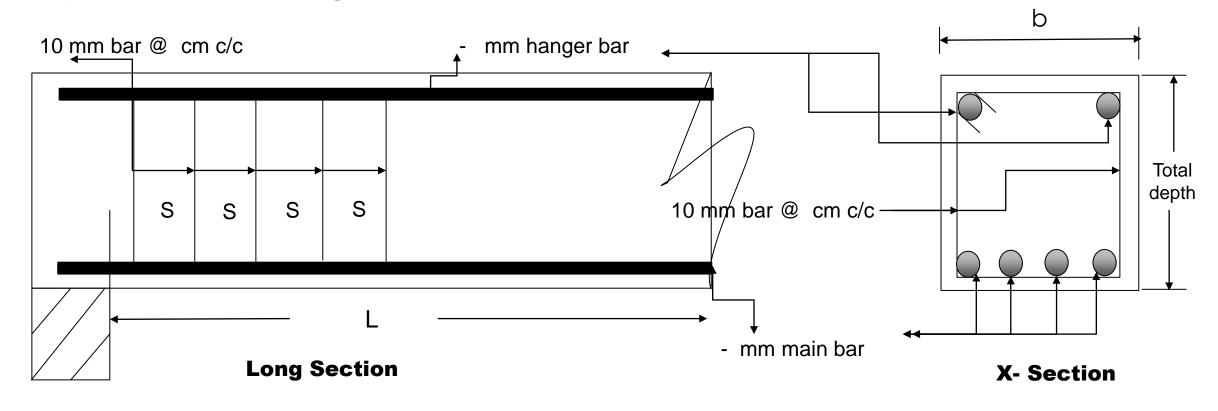
$$u_{all} = \frac{3 \cdot 23 \sqrt{f_C'}}{D}$$
 Allowable bond stress

If $u < u_{all}$

safe in bond.

Home work: 1) Memorize all formulas from step-1 to step-9, 2) understand all steps of rectangular beam design (WSD Method) and try to design a beam from a example in your books! Submit to you Teacher after opening the institutions!

Step-10: Detail drawing



Dear students, do you understand USD method?

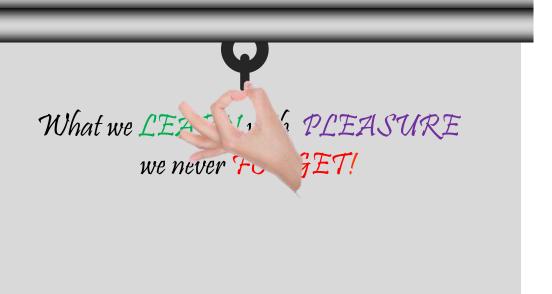
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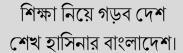
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Practice in your house & do the home works. welcome to next classes!







Design of Structure-1

6th semester (Civil), Chapter- 12, Lintel Design (WSD Method).Live class-15

Presented by

Md. Rezaul Bahar

Workshop Super & HoD (Civil)

Feni Polytechnic Institute, Feni. email- kajal.bahar@gmail.com

1) একটি বেড রুমের জানালার আকার 210 সেমি x 120 সেমি | জানালার উপর লিন্টেল ডিজাইন কর যখন উহার উপর 25 সেমি. প্রশস্থ দেওয়ালের উচ্চতা 2 মিটার এবং তথ্যাদি নিম্নরুপ -

 $f_c' = 210 \; k \; g/c \; m^2$, $f_s = 1400 \; k \; g/c \; m^2$, n = 10 , j = 0.88 , R = 15.50 , $v_c = 4.23 \; k \; g/c \; m^2$, $u_{all} = 16$



Step-1: Load Calculation

Hence, L = 2.10 + 0.15 = 2.25 m

Let,

Depth of Lintel = 15 cm

Width of Lintel = 25 cm

$$H = \frac{L}{2} \tan 60^{0} = \frac{2.25}{2} \tan 60^{0} = 1.95 < 2 \text{ m}$$
 (if H

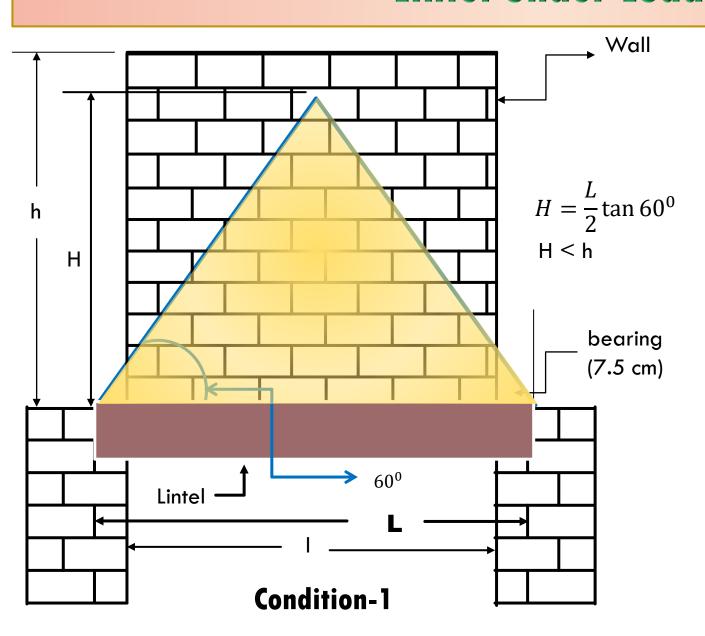
Weight of wall (Condition-1), $W_1 = 2.25 \times 0.25 \times 1.95 \times 1920 = 2106 \text{ kg}$

Self weight of Lintel (W_2) = 2.25 x 0.25 x 0.15 x 2400 = 203 kg

W = 2309 kg

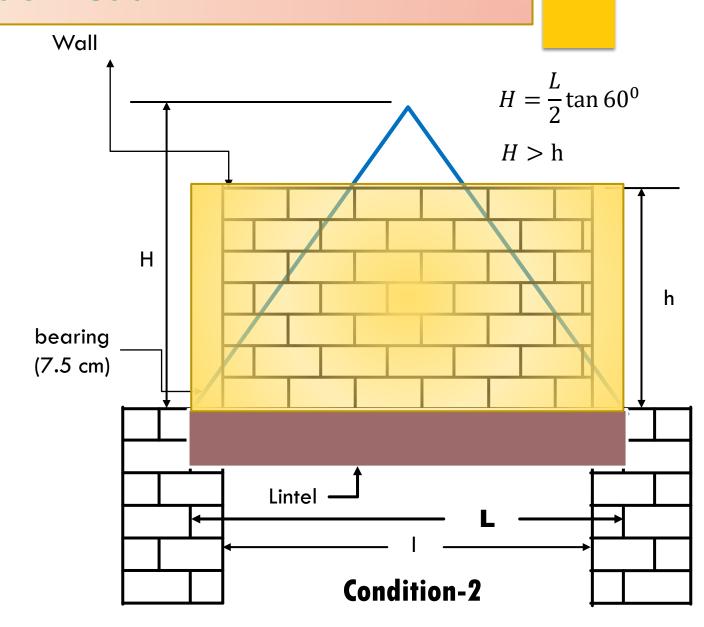


Lintel under Load





Lintel under Load





Step-2: $V_{\text{max}} = Maximum Shear force$

$$V = \frac{W}{2} = \frac{2309}{2} = 1155 \text{ kg}$$

Step-3: $M_{\text{max}} = Maximum Bending Moment$

For the weight of wall, $M_1 = \frac{W_1 L}{6} = \frac{2106 \times 2.25}{6} = 78975 \text{ kg.cm}$

For the weight of Lintel, $M_2 = \frac{W_2L}{8} = \frac{203 \times 2 \cdot 25}{8} = 5710$ kg.cm

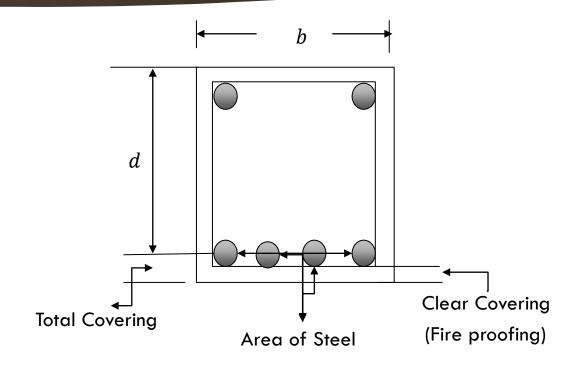
Total moment, $M = M_1 + M_2 = 78975 + 5710 = 84685 \text{ kg.cm}$

Step-4:
$$d = Effective depth$$

$$d = \sqrt{\frac{M}{Rb}} = \sqrt{\frac{78975}{15.5 \times 25}} = 14.93 \text{ cm}$$

Total depth = 14.93 + 4 = 18.93 cm Let, Clear covering = 3 cm

$$d = 15 - 4$$
 [here total covering = 4] $d = 11$ cm



Step-5: A_s = Area of Steel

$$A_S = \frac{M}{f_S j d} = \frac{84685}{1400 \times .87 \times 14.93} = 6.25 \ cm^2$$

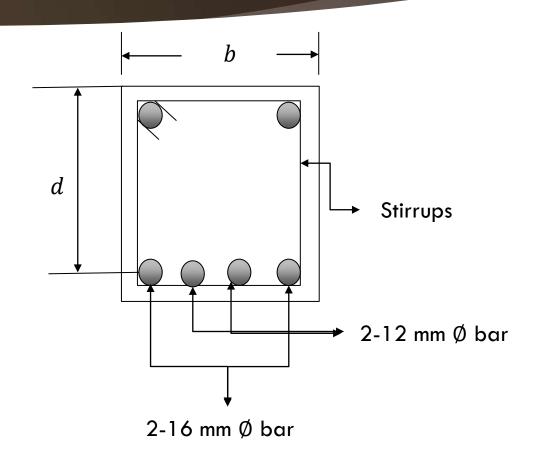
Find the total number of bars

By using area of bars.

By using 2-16mm + 2-12mm Ø bar

The area is 6.28 $cm^2 > 6.25 cm^2$

Use 2-16mm + 2-12mm Ø bar as main bar



Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{bd} = \frac{1042}{25 \times 11} = 3.79 \ k g/c m^2$$

Here,
$$V_{cr} = V - \frac{\omega d}{100} = 1155 - \frac{1026 \times 11}{100} = 1042 \text{ kg}$$

But allowable stress of concrete is, $v_C = 4.23 \ k \ g/c \ m^2$ $v < v_c$ stirrups not required.

So use 10 mm Ø bar@ 11 cm c/c as stirrups

Step-9: Check for Bond stress, u

$$u = \frac{V}{\sum 0jd} = \frac{1155}{17.58 \times .88 \times 11} = \underline{6.80 \ k \ g/c \ m^2}$$

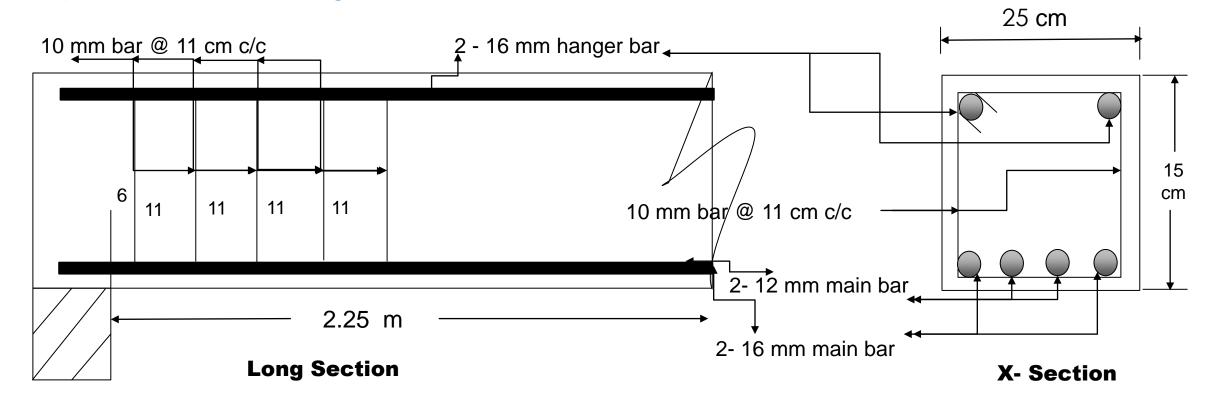
$$\sum 0 = N\pi D = 2 \times 3 \cdot 14 \times 1.6 + 2 \times 3 \cdot 14 \times 1.2 = 17.58$$

$$u_{all} = 16 \ k \ g/c \ m^2$$

$$u < u_{all}$$

safe in bond.

Step-10: Detail drawing



Home Work

- 1) একটি রুমের দরজার আকার 120 সেমি x 210 সেমি | দরজার উপর লিন্টেল ডিজাইন কর যখন উহার উপর 25 সেমি. প্রশস্থ দেওয়ালের উচ্চতা 1.5 মিটার এবং তথ্যাদি নিম্নরুপ
 - $f_c' = 211 \; k \; g/c \; m^2$, $f_s = 1450 \; k \; g/c \; m^2$, n = 9 , $v_c = 4.23 \; k \; g/c \; m^2$, $u_{all} = 16 \; k \; g/c \; m^2$
- 2) একটি রুমের জানালার আকার 210 সেমি x 120 সেমি | জানালার উপর লিন্টেল ডিজাইন কর যখন উহার উপর 25 সেমি. প্রশস্থ দেওয়ালের উচ্চতা 1.5 মিটার এবং তথ্যাদি নিম্নরুপ
 - $f_c' = 200 \; k \; g/c \; m^2$, $f_s = 1400 \; k \; g/c \; m^2$, n = 10 , $v_c = 4.23 \; k \; g/c \; m^2$, $u_{all} = 16 \; k \; g/c \; m^2$
- 3) একটি রুমের দরজার আকার 150 সেমি x 210 সেমি | দরজার উপর লিন্টেল ডিজাইন কর যখন উহার উপর 25 সেমি. প্রশস্থ দেওয়ালের উচ্চতা 2 মিটার এবং তথ্যাদি নিম্নরুপ
 - $f_c' = 211 \; k \; g/c \; m^2$, $f_s = 1450 \; k \; g/c \; m^2$, n=9 , $v_c=4.23 \; k \; g/c \; m^2$, $u_{all}=16 \; k \; g/c \; m^2$

Dear students, do you understand USD method?

if any questions then comment here & contact with me!

আর -

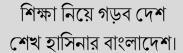
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"An investment in KNOWLEDGE pays the best INTEREST!"

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Design of Structure-1

6th semester (Civil), Chapter- 12, Lintel Design (WSD Method)

Presented by

Md. Rezaul Bahar

Workshop Super & HoD (Civil)

Feni Polytechnic Institute, Feni. email- kajal.bahar@gmail.com

2) একটি রুমের জানালার আকার 210 সেমি. x 120 সেমি. । জানালার উপর লিন্টেল ডিজাইন কর যখন উহার উপর 25 সেমি. প্রশস্থ দেওয়ালের উচ্চতা 1.0 মিটার এবং তথ্যাদি নিম্নরুপ -

 $f_c' = 211 \; k \; g/c \; m^2 \; , f_s = 1400 \; k \; g/c \; m^2 \; , j = 0.87 \; , R = 15.5 \; v_c = 4.23 \; k \; g/c \; m^2 \; , u_{all} = 16$



Step-1: Load Calculation

Hence, L = 2.10 + 0.15 = 2.25 m

Let,

Depth of Lintel = 15 cm

Width of Lintel = 25 cm

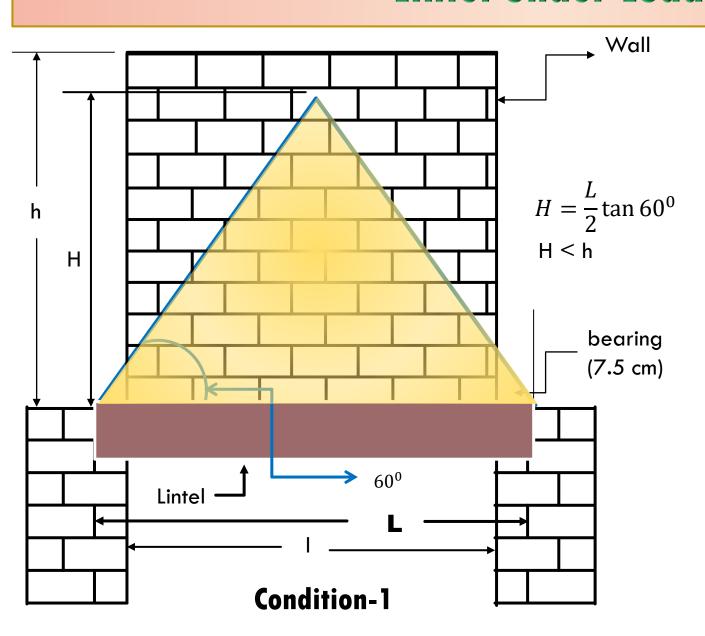
$$H = \frac{L}{2} \tan 60^{0} = \frac{2.25}{2} \tan 60^{0} = \underline{1.125} < \underline{1.0 \text{ m}}$$
 (if H>h then condition-2)

Weight of wall (Condition-2), $W_1 = 2.25 \times 1.0 \times 0.25 \times 1920 = 1080 \text{ kg}$

Self weight of Lintel (W_2) = 2.25 x 0.25 x 0.15 x 2400 = 203 kg

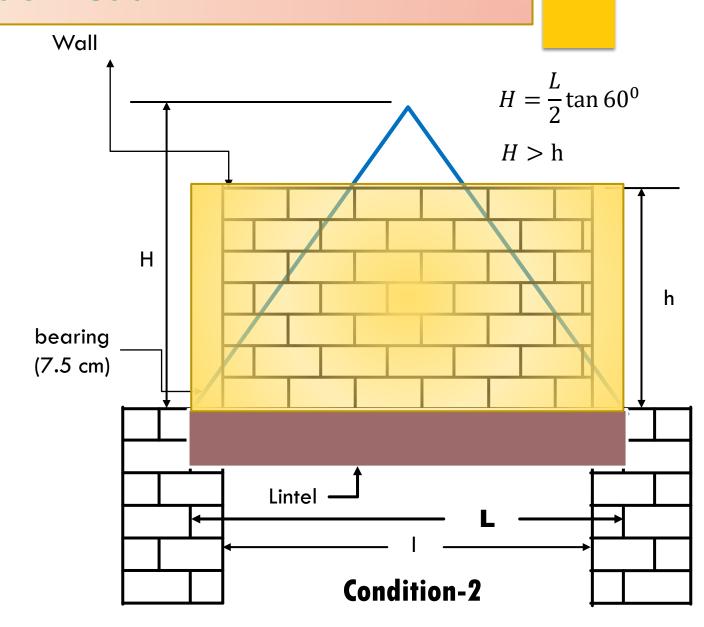
W = 1283 kg

Lintel under Load





Lintel under Load





Step-2: $V_{\text{max}} = Maximum Shear force$

$$V = \frac{W}{2} = \frac{1283}{2} = 642 \text{ kg}$$

Step-3: $M_{\text{max}} = Maximum Bending Moment$

For the weight of wall, $M_1 = \frac{W_1L}{6} = \frac{1080 \times 2 \cdot 25}{6} = 40500 \text{ kg.cm}$

For the weight of Lintel, $M_2 = \frac{W_2L}{8} = \frac{203 \times 2 \cdot 25}{8} = 5710 \text{ kg.cm}$

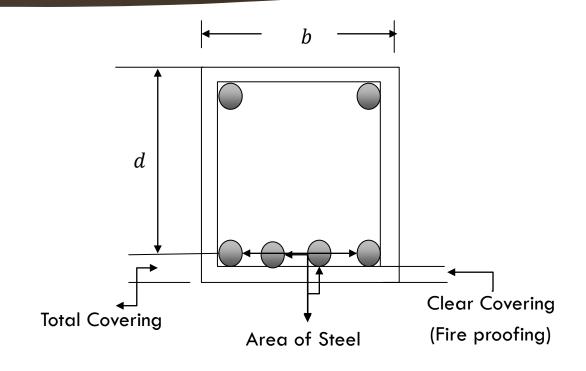
Total moment, $M = M_1 + M_2 = 40500 + 5710 = 46210 \text{ kg.cm}$

Step-4:
$$d = Effective depth$$

$$d = \sqrt{\frac{M}{Rb}} = \sqrt{\frac{46210}{15.5 \times 25}} = 10.95 \text{ cm}$$

Total depth =
$$10.95 + 4 = 14.95$$
 cm
Let, Clear covering = 3 cm

$$d = 15 - 4$$
 [here total covering = 4] $d = 11$ cm



Step-5: A_s = Area of Steel

$$A_S = \frac{M}{f_S j d} = \frac{46210}{1400 \times 0.87 \times 11} = 3.45 \text{ cm}^2$$

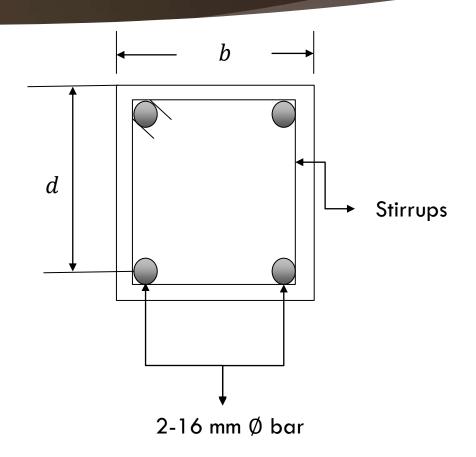
Find the total number of bars

By using area of bars.

By using 2-16 mm Ø bar

The area is $4.02 \ cm^2 > 3.45 \ cm^2$

Use 2-16mm Ø bar as main bar.



Step - 6: Check for Shear stress, v

$$v = \frac{V_{cr}}{bd} = \frac{580}{25 \times 11} = 2.10 \ k \ g/c \ m^2$$

Here,
$$V_{cr} = V - \frac{\omega d}{100} = 642 - \frac{1283 \times 11}{2.25 \times 100} = 580 \text{ kg}$$

But allowable stress of concrete is, $v_C = 4.23 \ k \ g/c \ m^2$

 $v < v_c$ stirrups not required.

So use 10 mm Ø bar@ 11 cm c/c as stirrups

Step-9: Check for Bond stress, u

$$u = \frac{V}{\sum 0jd} = \frac{642}{10.05 \times .88 \times 11} = \underline{6.60 \ k \ g/c \ m^2} \qquad \sum 0 = N\pi D = 2 \times 3 \cdot 14 \times 1.6 = 10.05$$

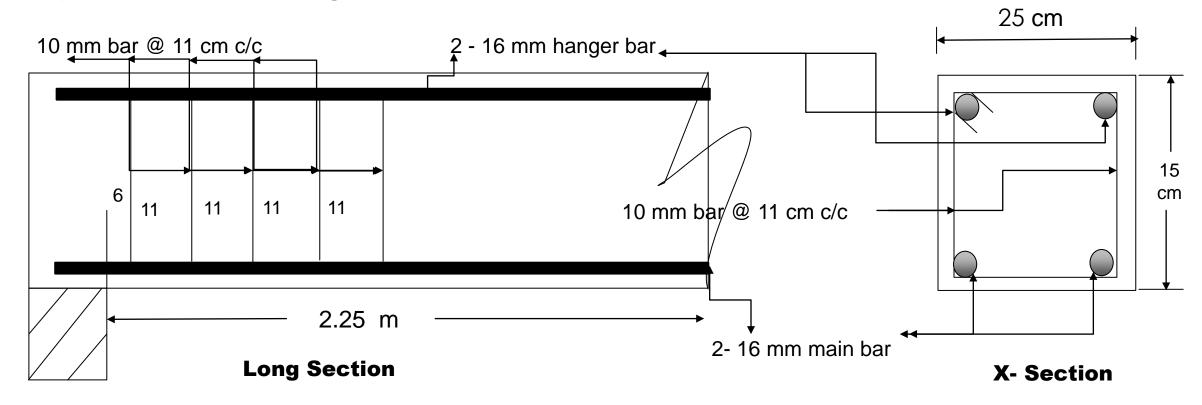
$$u_{all} = 16 k g/c m^2$$

$$u < u_{all}$$

safe in bond.

$$\sum 0 = N\pi D = 2 \times 3 \cdot 14 \times 1.6 = 10.05$$

Step-10: Detail drawing



Home Work

- 1) একটি রুমের দরজার আকার 120 সেমি x 210 সেমি | দরজার উপর লিন্টেল ডিজাইন কর যখন উহার উপর 25 সেমি. প্রশস্থ দেওয়ালের উচ্চতা 1.5 মিটার এবং তথ্যাদি নিম্নরুপ
 - $f_c' = 211 \; k \; g/c \; m^2$, $f_s = 1450 \; k \; g/c \; m^2$, n = 9 , $v_c = 4.23 \; k \; g/c \; m^2$, $u_{all} = 16 \; k \; g/c \; m^2$
- 2) একটি রুমের জানালার আকার 210 সেমি x 120 সেমি | জানালার উপর লিন্টেল ডিজাইন কর যখন উহার উপর 25 সেমি. প্রশস্থ দেওয়ালের উচ্চতা 1.5 মিটার এবং তথ্যাদি নিম্নরুপ
 - $f_c' = 200 \; k \; g/c \; m^2$, $f_s = 1400 \; k \; g/c \; m^2$, n = 10 , $v_c = 4.23 \; k \; g/c \; m^2$, $u_{all} = 16 \; k \; g/c \; m^2$
- 3) একটি রুমের দরজার আকার 150 সেমি x 210 সেমি | দরজার উপর লিন্টেল ডিজাইন কর যখন উহার উপর 25 সেমি. প্রশস্থ দেওয়ালের উচ্চতা 2 মিটার এবং তথ্যাদি নিম্নরুপ
 - $f_c' = 211 \ k \ g/c \ m^2$, $f_s = 1450 \ k \ g/c \ m^2$, n = 9 , $v_c = 4.23 \ k \ g/c \ m^2$, $u_{all} = 16 \ k \ g/c \ m^2$

Dear students, do you understand USD method?

if any questions then comment here & contact with me!

আর -

এই ভিডিওটি পুণরায় দেখতে দক্ষতা বাতায়ন বা www.facebook.com/skills.gov.bd

ভিজিট কর !

"An investment in KNOWLEDGE pays the best INTEREST!"

Practice in your house & do the home works. welcome to next classes!

